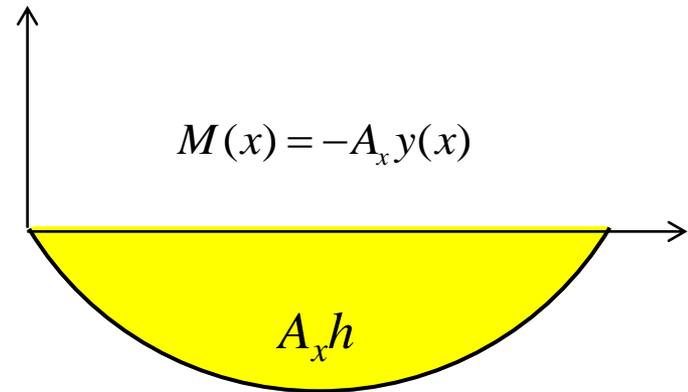
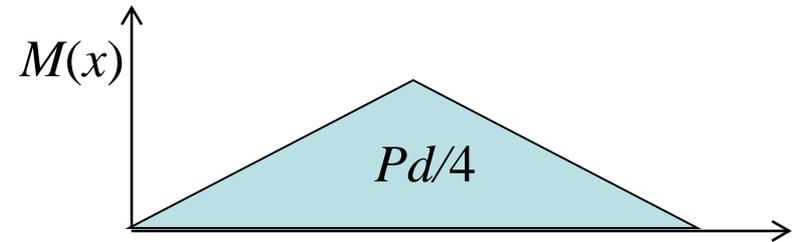
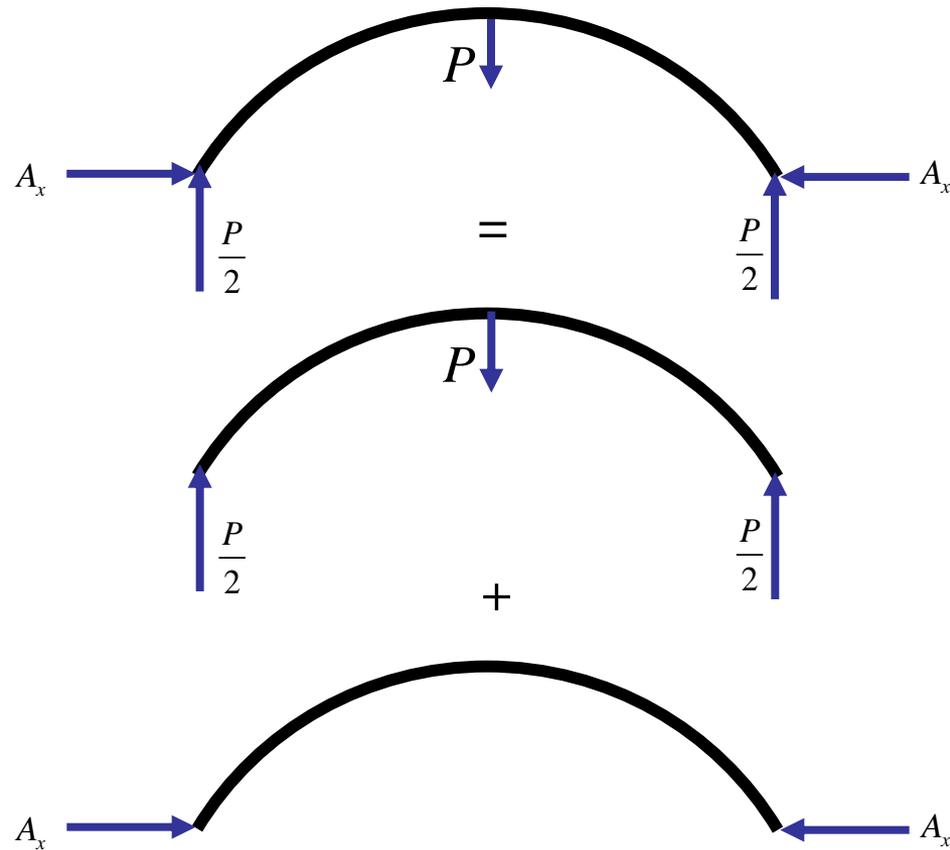
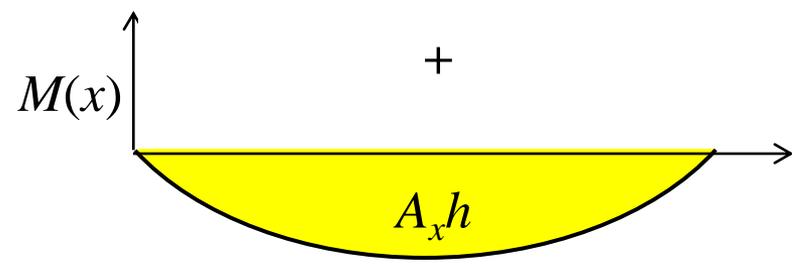
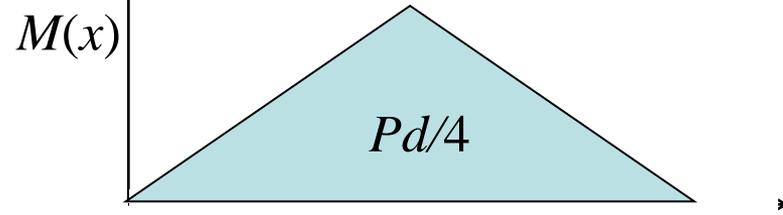
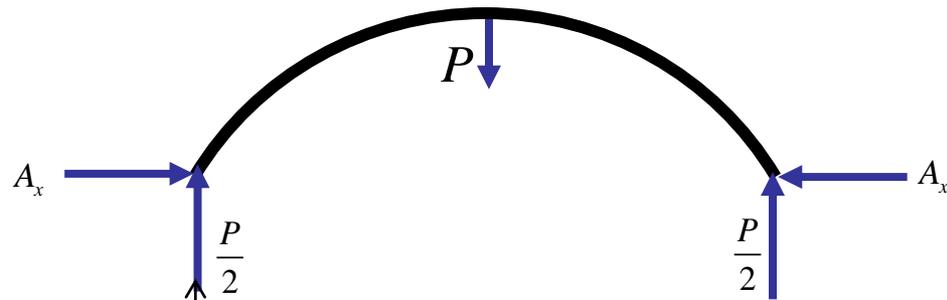


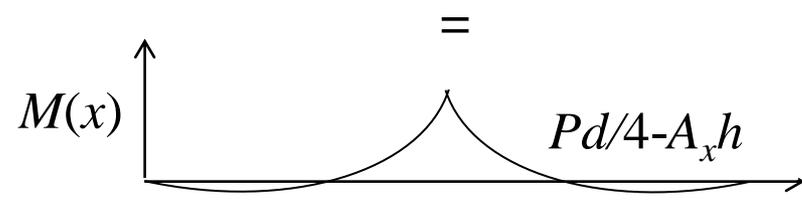
Approximate Analysis: Point Load



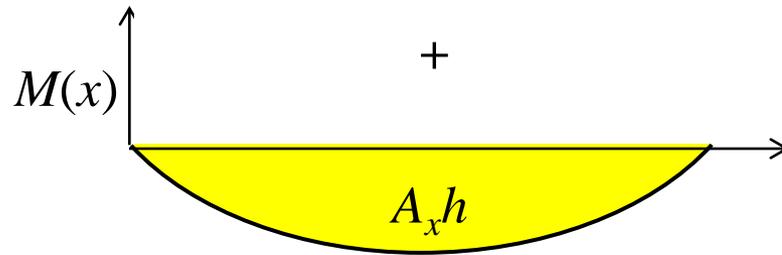
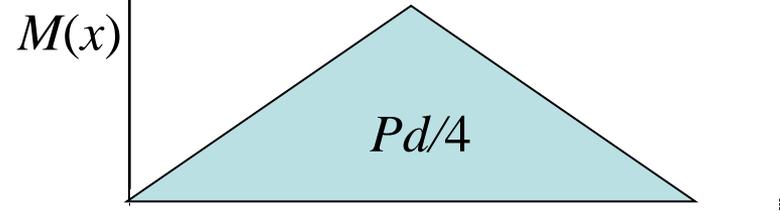
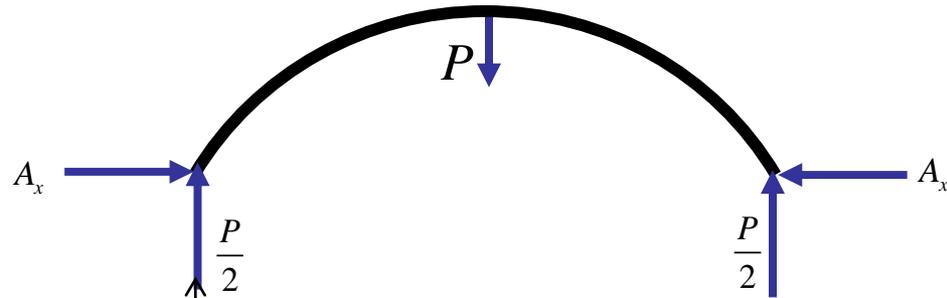
Moments



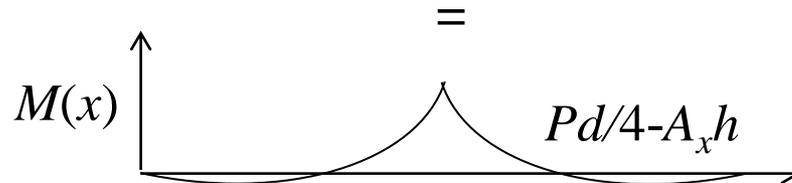
$$M(x) = -A_x y(x)$$



Moments

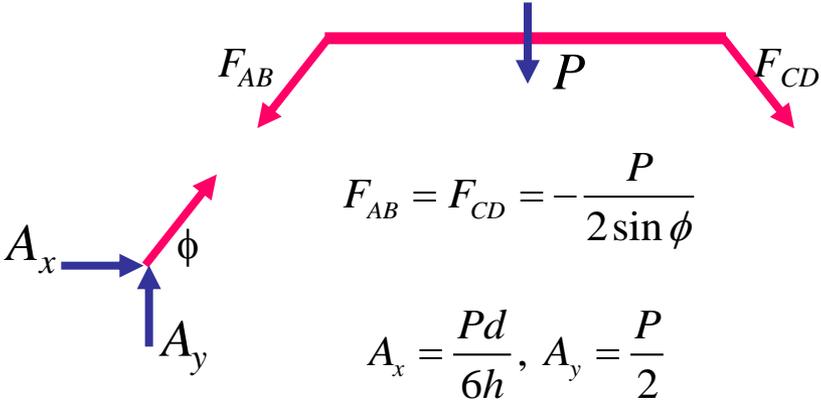
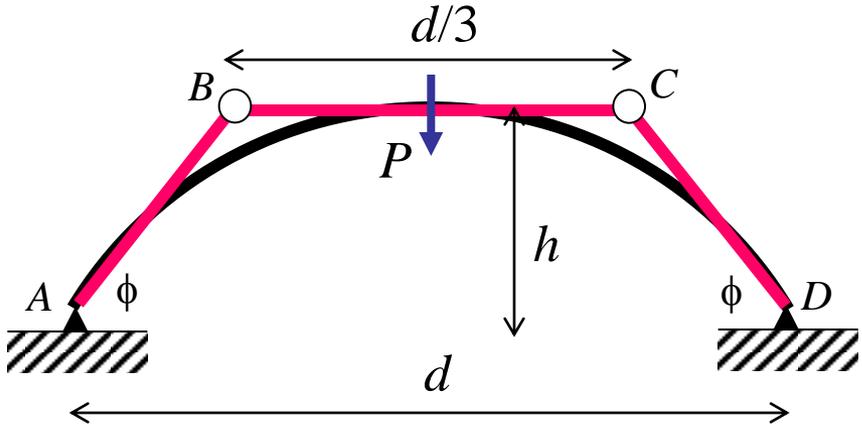
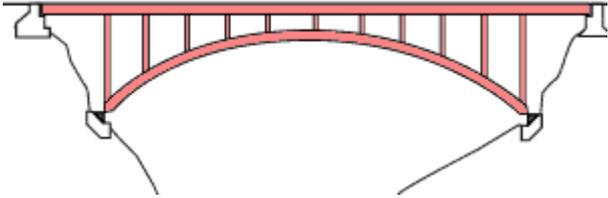


$$M(x) = -A_x y(x)$$



Typically: $0 \leq A_x \leq \frac{P d}{4 h}$

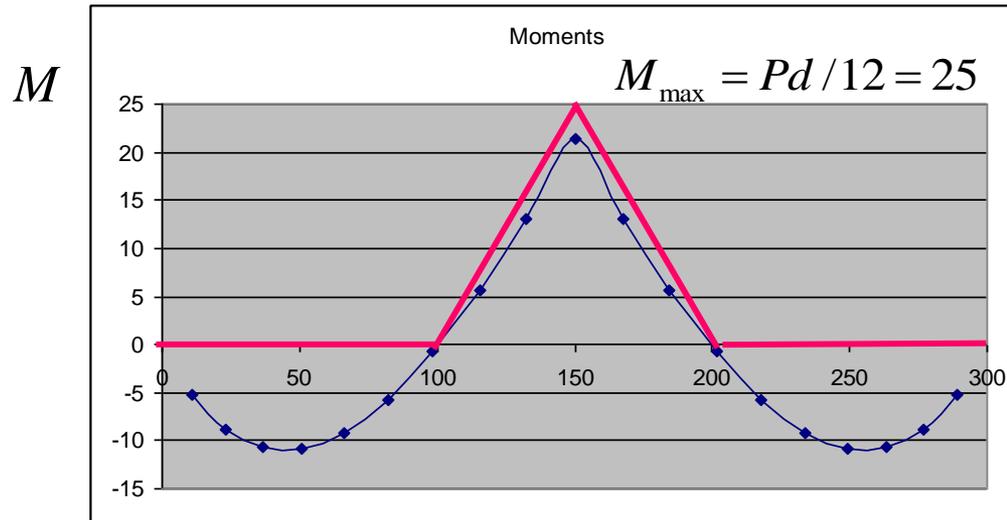
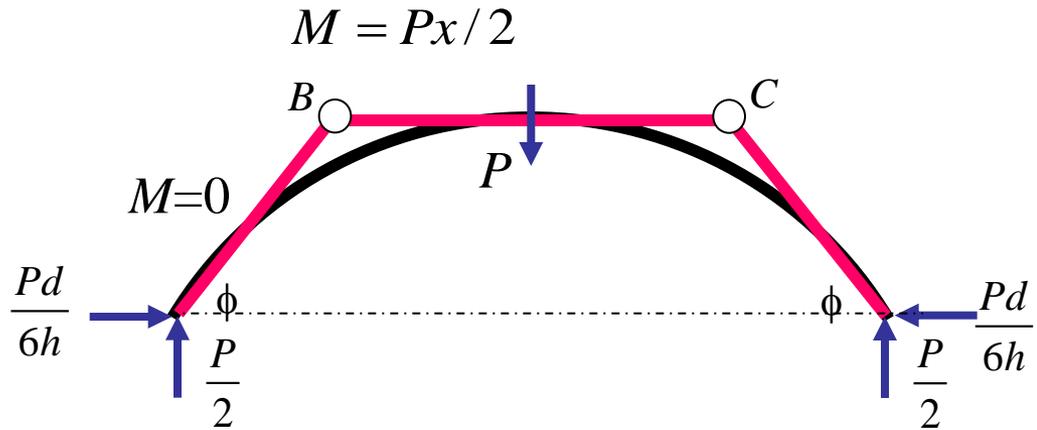
Approximate Analysis



$$F_{AB} = F_{CD} = -\frac{P}{2 \sin \phi}$$

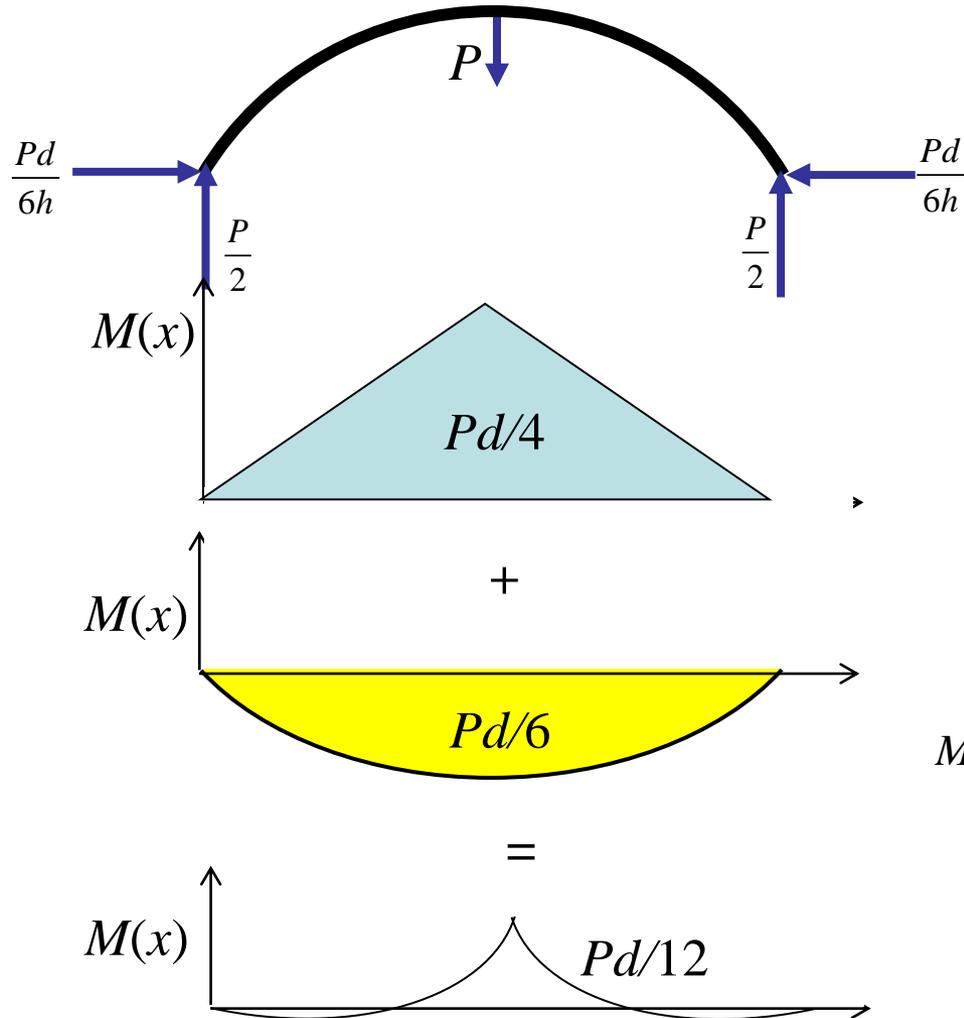
$$A_x = \frac{Pd}{6h}, \quad A_y = \frac{P}{2}$$

Moments (1)



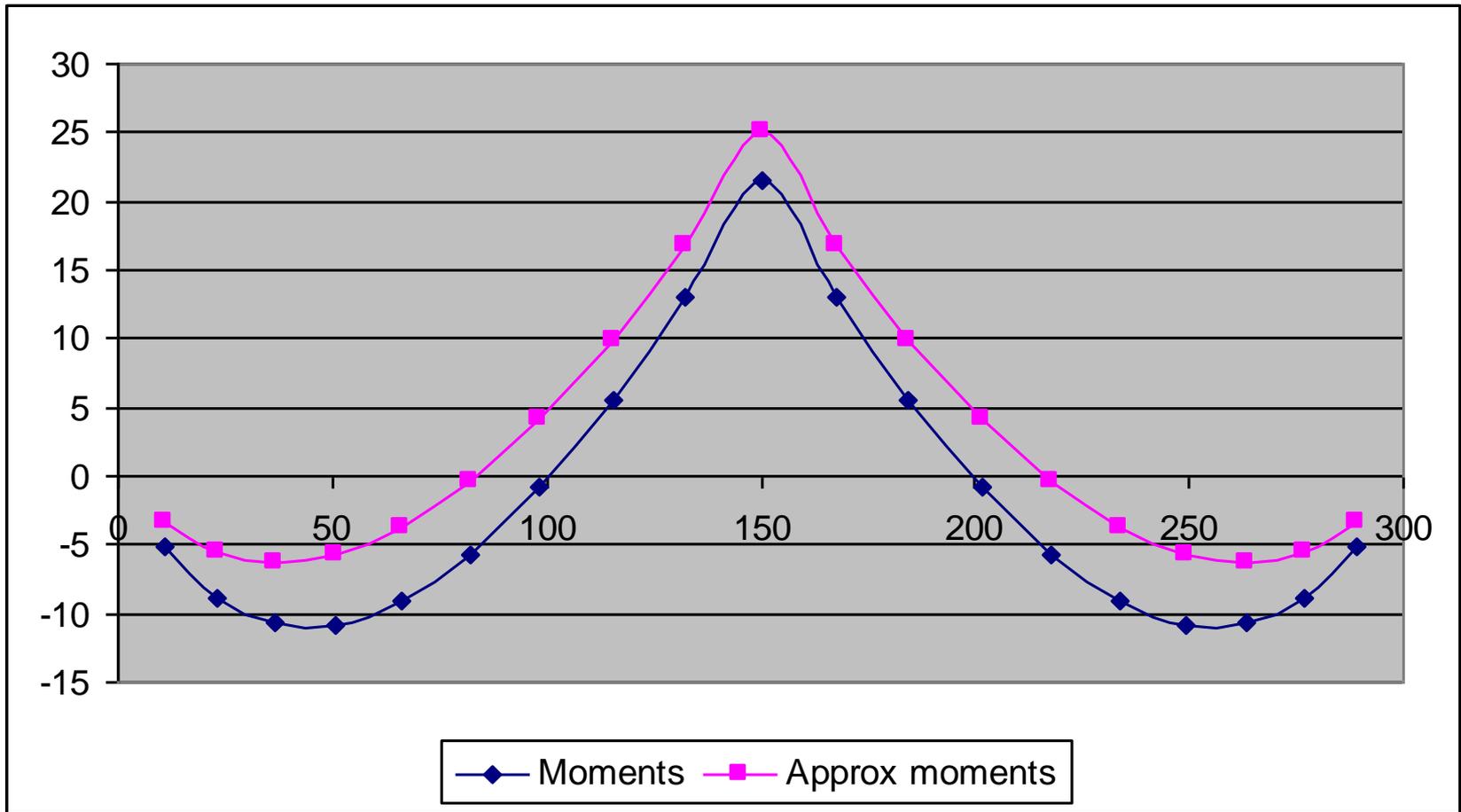
For $E=10^6$, $P=1$, $d=300\text{ft}$, $h=75\text{ft}$, $I=0.0833\text{ft}^4$, $A=1\text{ft}^2$

Moments (2)

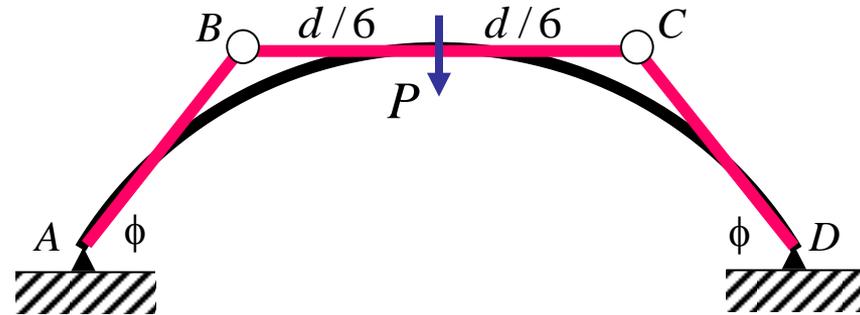


$$M(x) = -\frac{Pd}{6h} y(x)$$

P=1, d=300ft, h=75ft



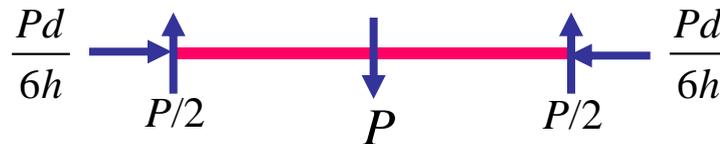
Deflections



$$\tan \phi = \frac{3h}{d},$$

$$L_{AB} = \sqrt{h^2 + d^2/9}$$

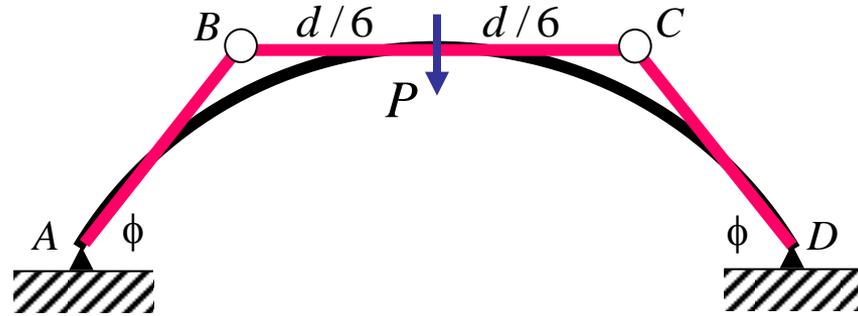
$$\delta_{AB} = \frac{F_{AB} L_{AB}}{EA} = \mathbf{u}_B \cdot \mathbf{n}_{AB} = u_x^B \cos \phi + u_y^B \sin \phi = -\frac{P L_{AB}}{2EA \sin \phi}.$$



$$u_x^B = -\frac{Pd}{6h} \frac{d}{6EA} = \frac{Pd^2}{36hEA}$$

$$u_y^P = u_y^B - \frac{Pd/3}{48EI}$$

Deflections



$$\tan \phi = \frac{3h}{d}$$

$$\sin \phi = \frac{h}{L_{AB}}$$

$$L_{AB} = \sqrt{h^2 + d^2 / 9}$$

$$\frac{Pd^2}{36hEA} \cos \phi + u_y^B \sin \phi = -\frac{PL_{AB}}{2EA \sin \phi}$$

$$\Rightarrow u_y^B = -\frac{PL_{AB}}{2EA \sin^2 \phi} - \frac{Pd^2}{36EAh \tan \phi} = -\frac{P(h^2 + d^2 / 9)^{3/2}}{2EAh^2} - \frac{Pd^3}{108EAh^2}$$

$$u_y^P = u_y^B - \frac{P(d/3)^3}{48EI}$$

For $E=10^6$, $P=1$, $d=300\text{ft}$, $h=75\text{ft}$, $I=0.0833\text{ft}^4$, $A=1\text{ft}^2$

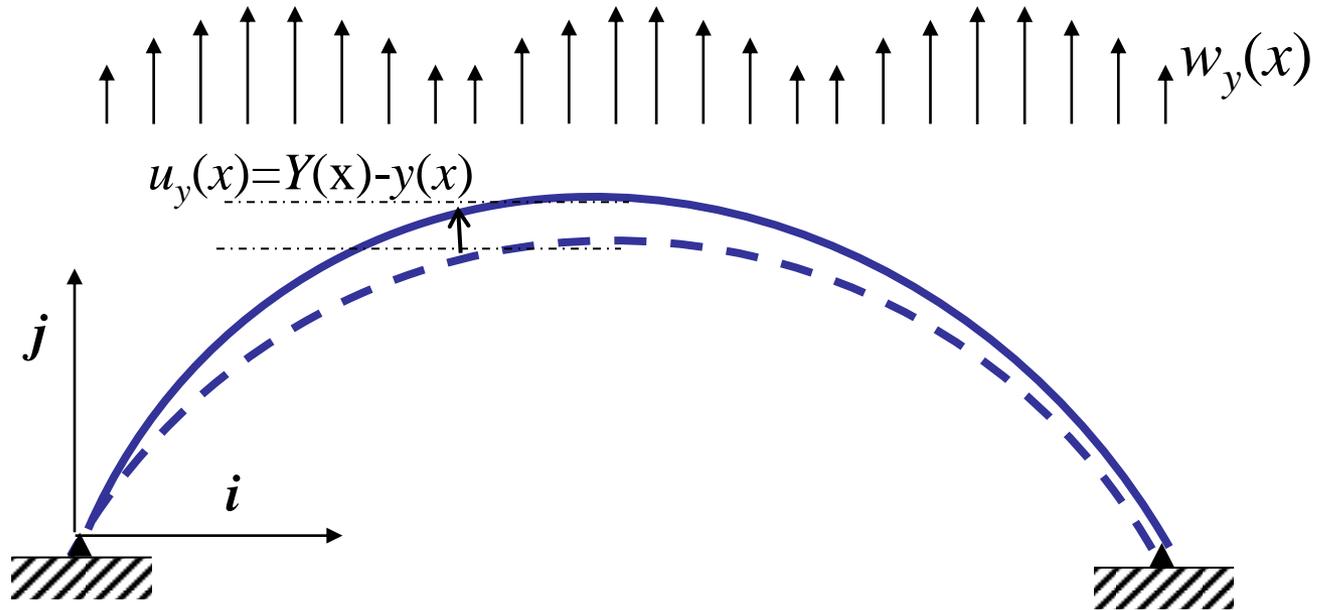
$$u_y^P = -0.25\text{ft}$$

Actual: 0.28 feet

Arch Deflections: Energy Methods

$y=y(x)$ (undef. geometry)

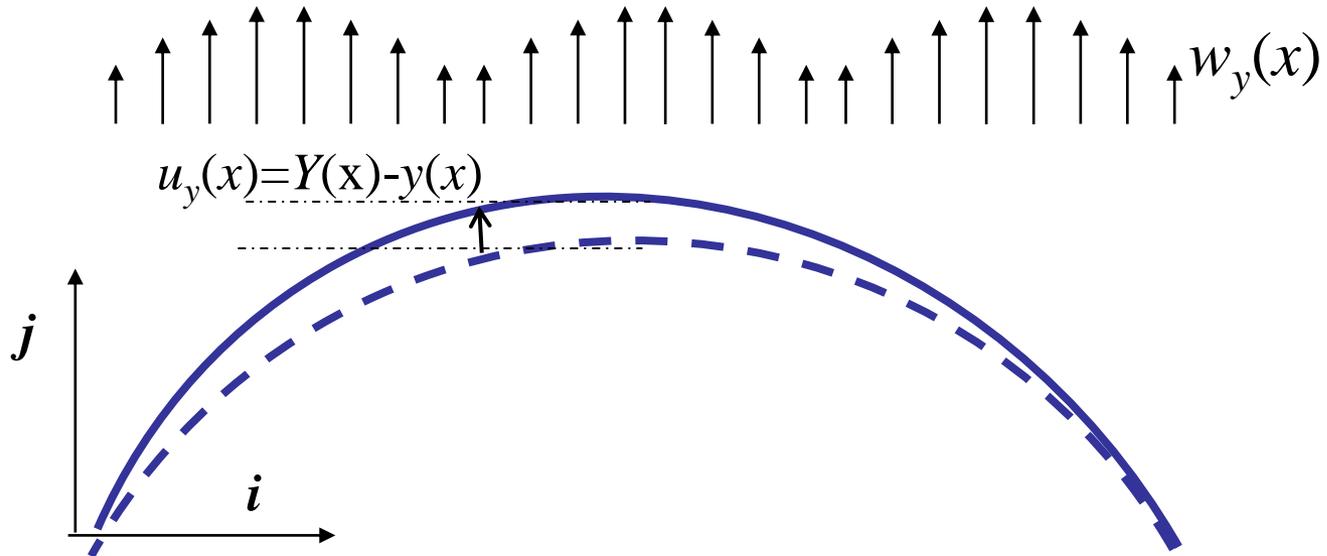
$Y=Y(X)$ (deformed geometry)



Energy due to the load w_y :

$y=y(x)$ (undef. geometry)

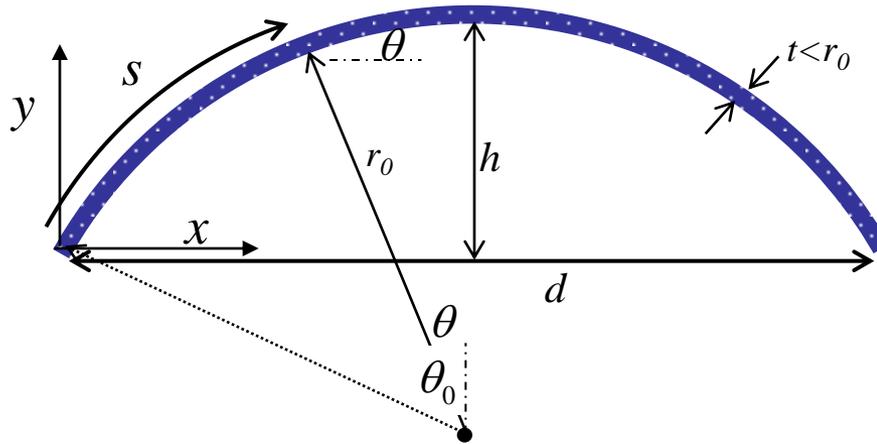
$Y=Y(X)$ (deformed geometry)



$$V^{\text{load}} = -\int w_y(x)u_y(x)dx$$

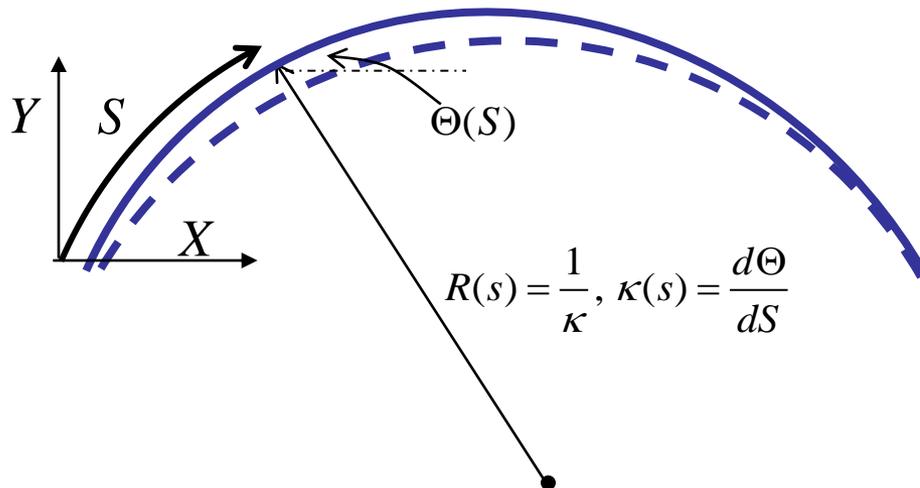
Strain Energy

Undeformed geometry $y=y(x)$: constant curvature κ_0



$$\theta(s) = \theta_0 - \frac{s}{r_0}; \kappa_0 = \frac{d\theta}{ds} = -\frac{1}{r_0}$$

Deformed geometry: $Y=Y(X)$ Curvature $\kappa(s)$



$$R(s) = \frac{1}{\kappa}, \kappa(s) = \frac{d\Theta}{dS}$$

Moment is

$$M = EI(\kappa - \kappa_0)$$

Strain Energy is

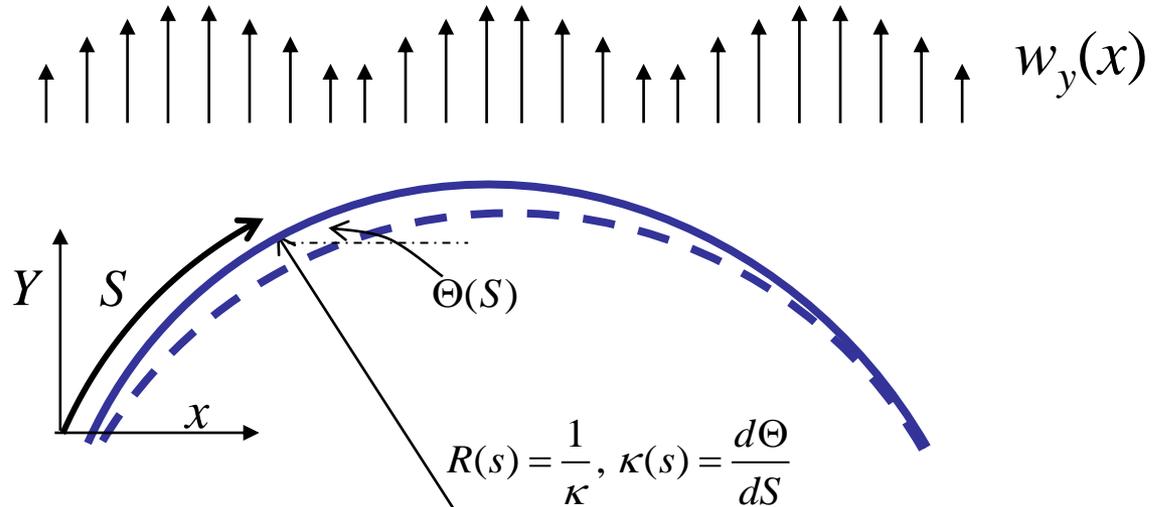
$$V = \int \frac{1}{2} EI (\kappa_0 - \kappa)^2 ds + \int \frac{1}{2} EA \left(\frac{dS}{ds} \right)^2 ds$$

Total Energy to be minimized

$y=y(x)$ (undef. geometry)

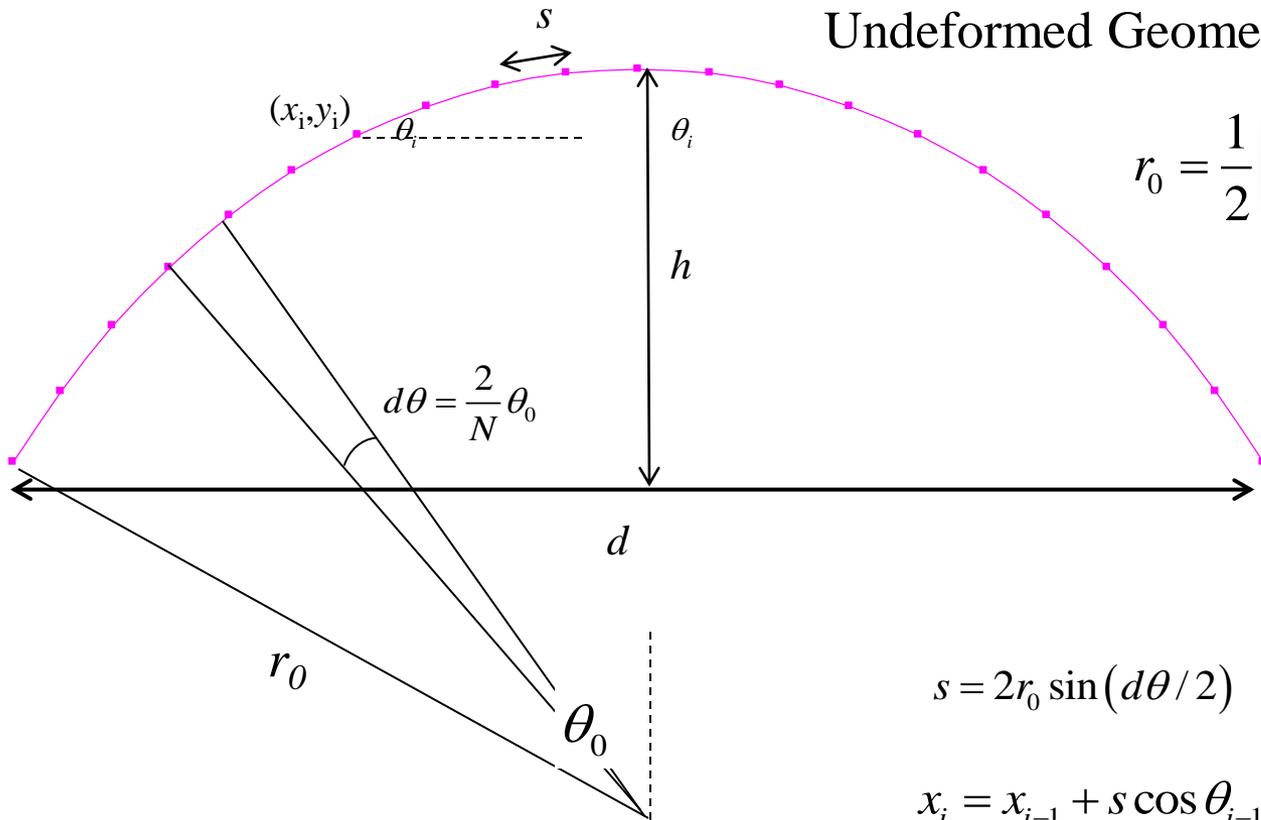
$Y=Y(X)$ (deformed geometry)

$$u_y(x) = Y(x) - y(x)$$



$$V = \int \frac{1}{2} EI (\kappa_0 - \kappa)^2 ds + \int \frac{1}{2} EI \left(\frac{dS}{ds} \right)^2 ds - \int w_y(x) u_y(x) dx$$

Undeformed Geometry: N segments



Undeformed Geometry

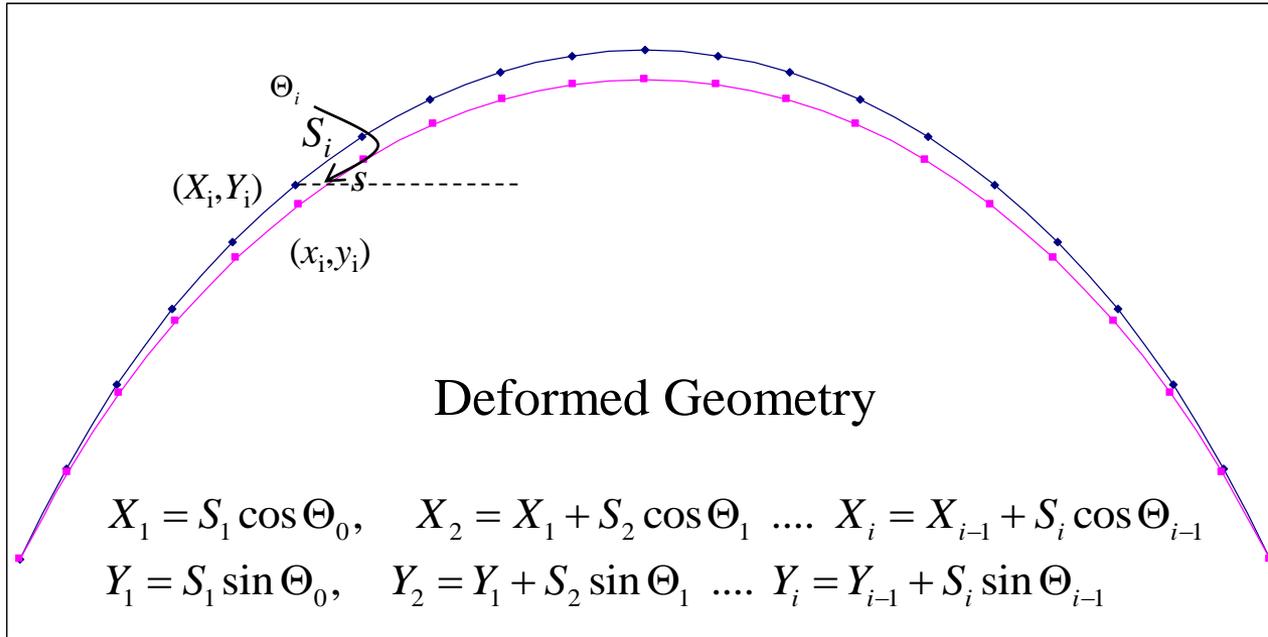
$$r_0 = \frac{1}{2} \left(\frac{d^2}{4h} + h \right)$$

$$\theta_0 = \tan^{-1} \frac{d}{2(r_0 - h)}$$

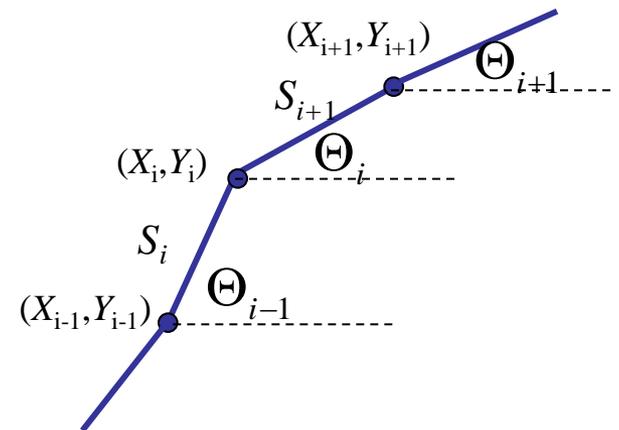
$$s = 2r_0 \sin(d\theta/2) \quad \theta_i = \theta_0 - d\theta(i+1/N)$$

$$x_i = x_{i-1} + s \cos \theta_{i-1}, \quad y_i = y_{i-1} + s \sin \theta_{i-1}$$

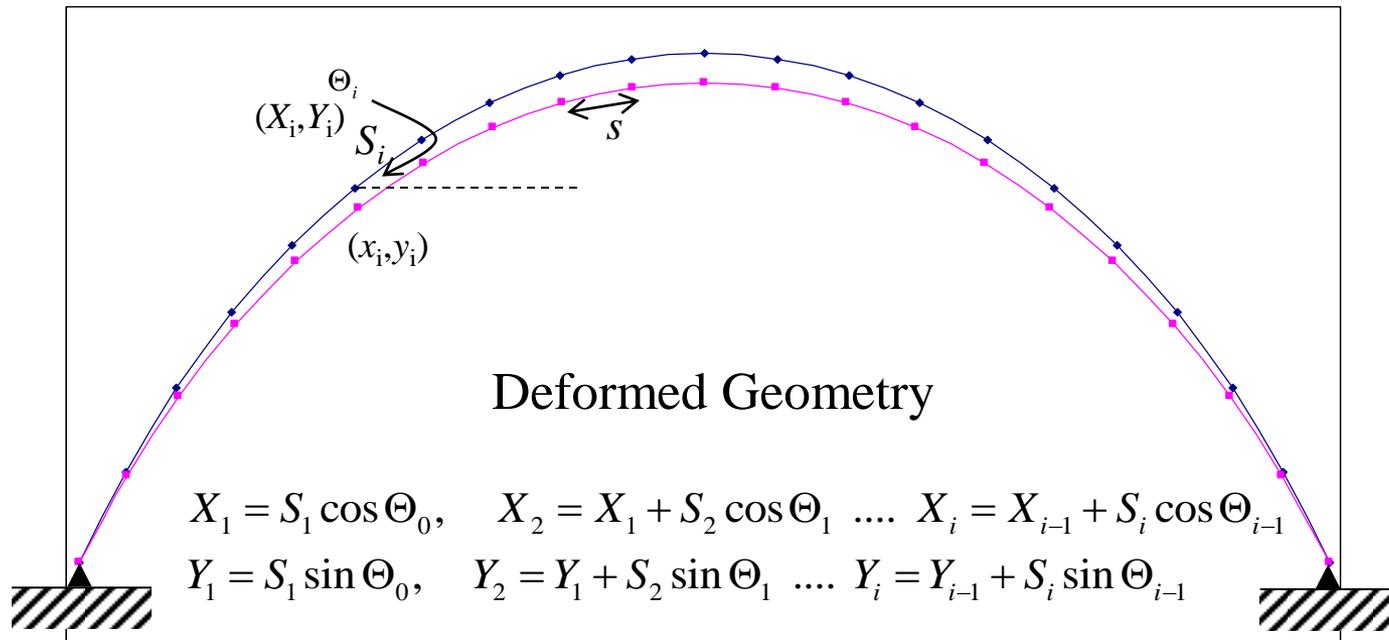
Ugly...



- Curvature in segment i is $\kappa_i = (\Theta_{i-1} - \Theta_i) / S_i$
- Moment at i is $M_i = EI(\kappa_i - \kappa_0)$ $\kappa_0 = 1/r_0$
- Axial force for segment i is $EA(s - S_i) / s$
- Deflection at point i is $u_y = Y_i - y_i$
- Load at i is W_i



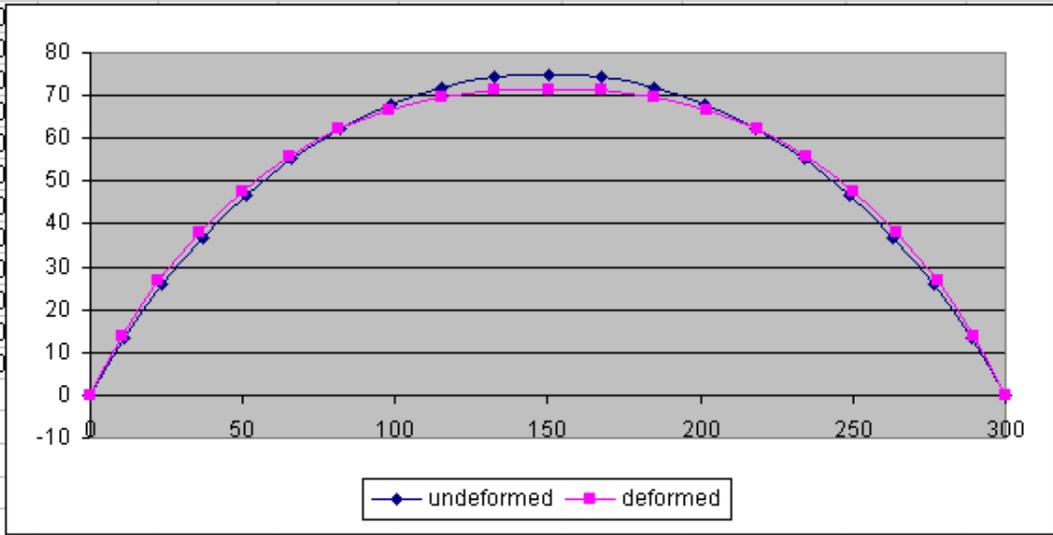
Minimize V , Varying Θ_i, S



$$V = \frac{1}{2} EI \sum_{i=1}^{N-1} s \left(\frac{1}{r_o} - \kappa_i \right)^2 + \frac{1}{2} EA \sum_{i=0}^{N-1} s \left(\frac{S_i - s}{s} \right)^2 - \sum_{i=0}^N W_i u_i$$

Constraints from BC.

| | | | | | | | | | | | | | | |
|----|------------------|----------|-----------|----------|----------|-------------------|----------|----------|-------------|------------------|-------------|------------|-----------|-------------|
| 4 | rise h | 75 | section i | 0.083333 | | center x | 150 | | | numb of segs | 20 | | anglespan | 0.927295218 |
| 5 | | | area | 1 | | center y | 112.5 | | | undef chorchleng | 17.38056 | | dtheta0 | 0.092729522 |
| 7 | initial geometry | | | | force P | deformed geometry | | | | | | | | |
| 8 | point | theta | x | y | S | theta | X | Y | curv k | delta k | PE bending | PE stretch | PE load | |
| 9 | 0 | 0.88093 | 0 | 0 | 17.38041 | 0.926362 | 0 | 0 | 0.005954964 | 0.000622 | 0.027984442 | 6.05E-05 | | |
| 10 | 1 | 0.788201 | 11.06157 | 13.40617 | 17.38041 | 0.822862 | 10.44122 | 13.89459 | 0.006390363 | 0.001057 | 0.080914604 | 6.02E-05 | 0 | |
| 11 | 2 | 0.695471 | 23.31699 | 25.73048 | 17.38041 | 0.711795 | 22.26209 | 26.63609 | 0.006620534 | 0.001287 | 0.119990017 | 5.99E-05 | 0 | |
| 12 | 3 | 0.602742 | 36.66094 | 36.86702 | 17.38041 | 0.596727 | 35.42238 | 37.98887 | 0.006636283 | 0.001302 | 0.122755532 | 5.98E-05 | 0 | |
| 13 | 4 | 0.510012 | 50.97877 | 46.72012 | 17.38041 | 0.481403 | 49.79909 | 47.75559 | 0.006433884 | 0.001101 | 0.087714734 | 6.01E-05 | 0 | |
| 14 | 5 | 0.417283 | 66.14745 | 55.2051 | 17.38041 | 0.36958 | 65.20415 | 55.80313 | 0.006025444 | 0.000692 | 0.034689904 | 6.04E-05 | 0 | |
| 15 | 6 | 0.324553 | 82.03664 | 62.24905 | 17.38041 | 0.264855 | 81.41102 | 62.08134 | 0.005427678 | 9.43E-05 | 0.000644597 | 6.04E-05 | 0 | |
| 16 | 7 | 0.231824 | 98.50982 | 67.79146 | 17.38041 | 0.17052 | 98.18539 | 66.631 | 0.004664922 | -0.00067 | 0.032354864 | 5.96E-05 | 0 | |
| 17 | 8 | 0.139094 | 115.4254 | 71.7847 | 17.38041 | 0.089442 | 115.3137 | 69.58036 | | | | | 0 | |
| 18 | 9 | 0.046365 | 132.6381 | 74.19444 | 17.380 | | | | | | | | 0 | |
| 19 | 10 | -0.04636 | 150 | 75 | -1 | 17.380 | | | | | | | 0 | |
| 20 | 11 | -0.13909 | 167.3619 | 74.19444 | 17.380 | | | | | | | | 0 | |
| 21 | 12 | -0.23182 | 184.5746 | 71.7847 | 17.380 | | | | | | | | 0 | |
| 22 | 13 | -0.32455 | 201.4902 | 67.79146 | 17.380 | | | | | | | | 0 | |
| 23 | 14 | -0.41728 | 217.9634 | 62.24905 | 17.380 | | | | | | | | 0 | |
| 24 | 15 | -0.51001 | 233.8525 | 55.2051 | 17.380 | | | | | | | | 0 | |
| 25 | 16 | -0.60274 | 249.0212 | 46.72012 | 17.380 | | | | | | | | 0 | |
| 26 | 17 | -0.69547 | 263.3391 | 36.86702 | 17.380 | | | | | | | | 0 | |
| 27 | 18 | -0.7882 | 276.683 | 25.73048 | 17.380 | | | | | | | | 0 | |
| 28 | 19 | -0.88093 | 288.9384 | 13.40617 | 17.380 | | | | | | | | 0 | |
| 29 | 20 | -0.97366 | 300 | -1.8E-14 | 17.380 | | | | | | | | 0 | |



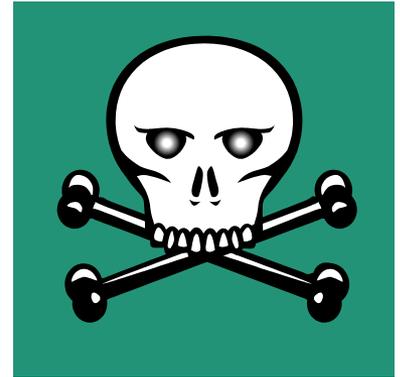
Don't Worry, Be Happy

You don't have
to write your
own solver.

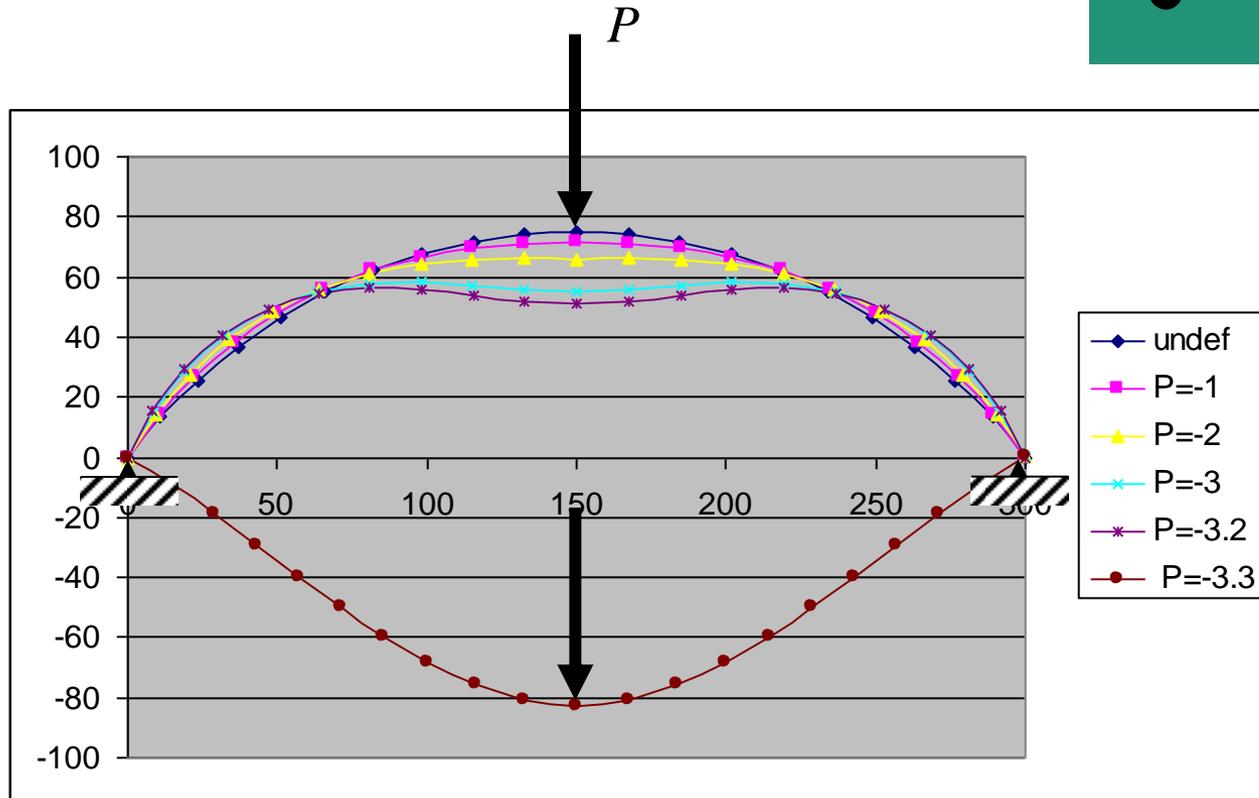
You can have
ours.



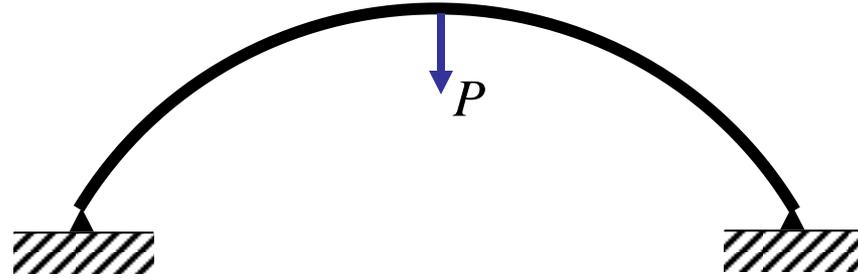
Beware Snap-through Buckling!



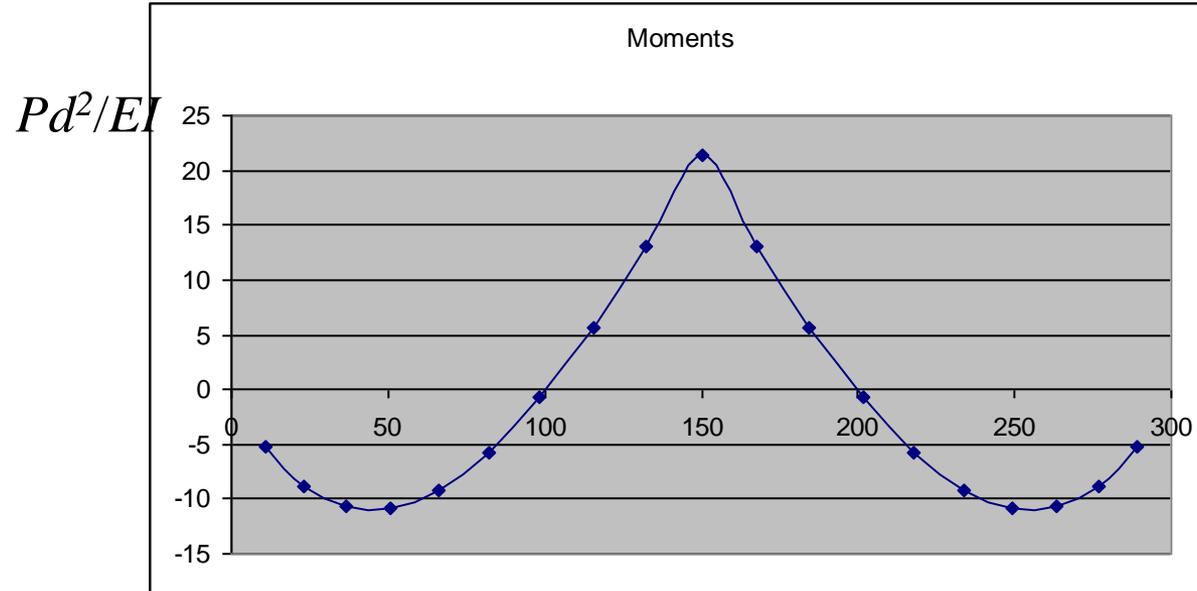
$$E=10^5, I=1/12, A=1, d=300, h=70$$



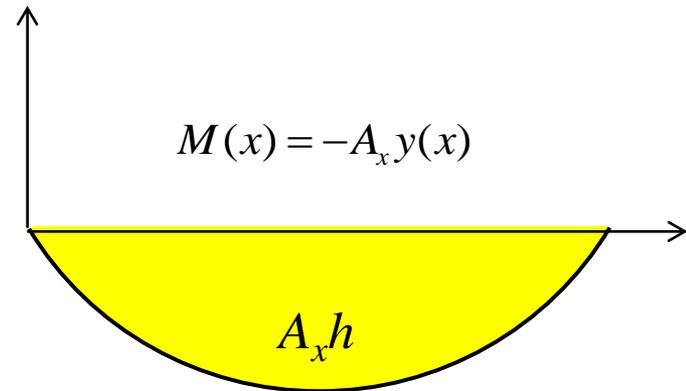
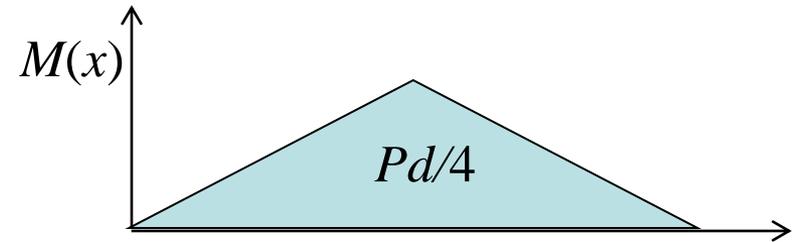
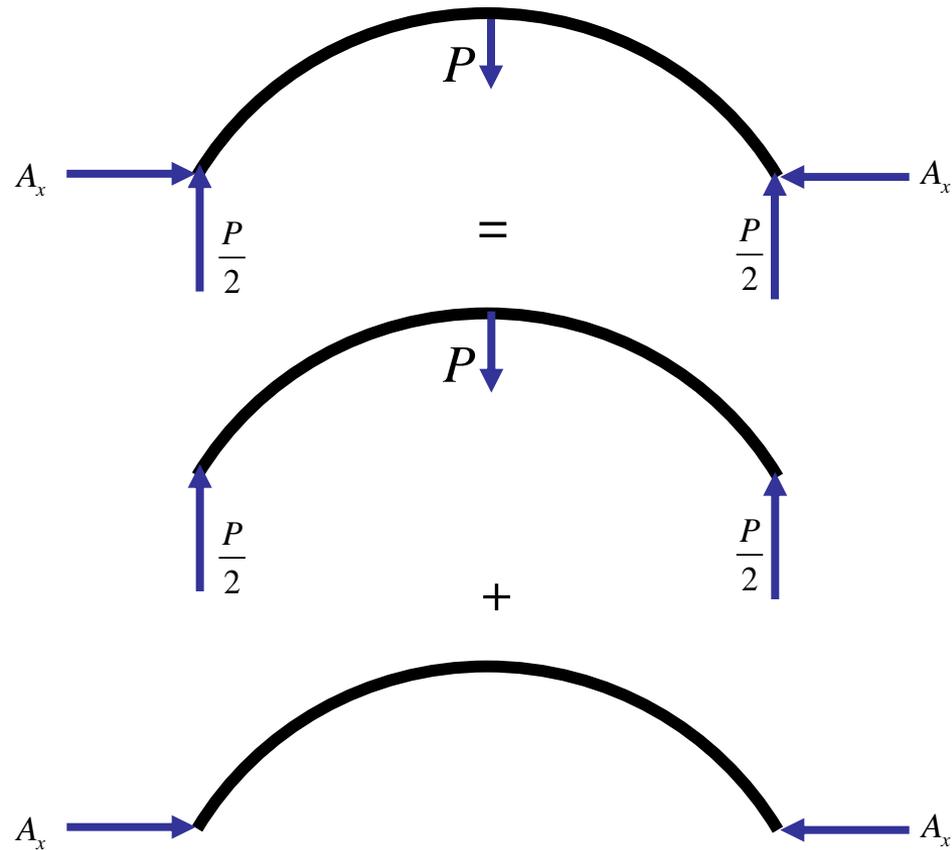
Small deflections not assumed!



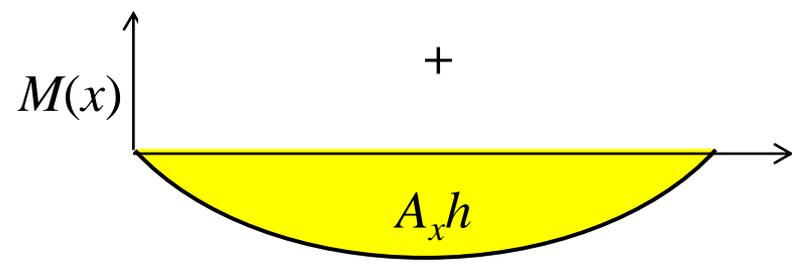
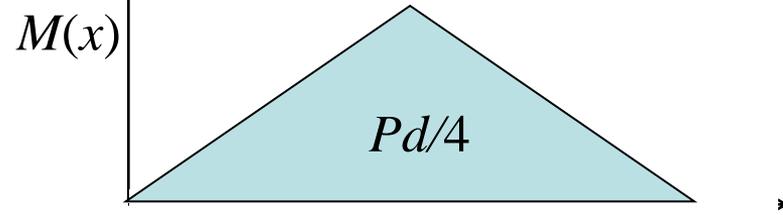
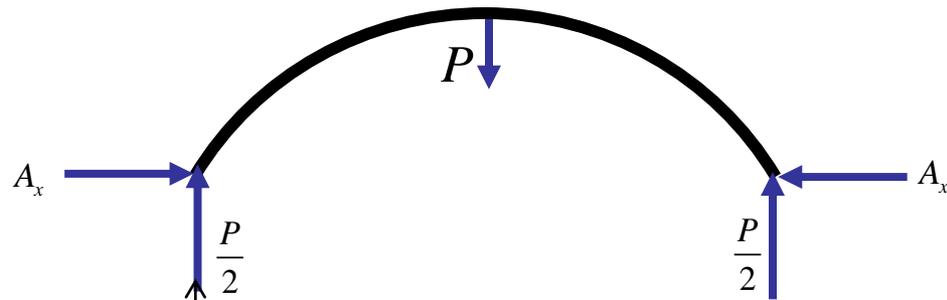
Moments



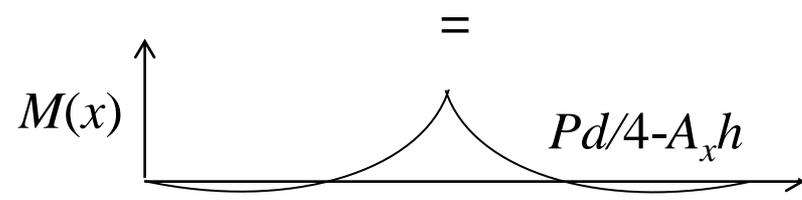
Approximate Moments



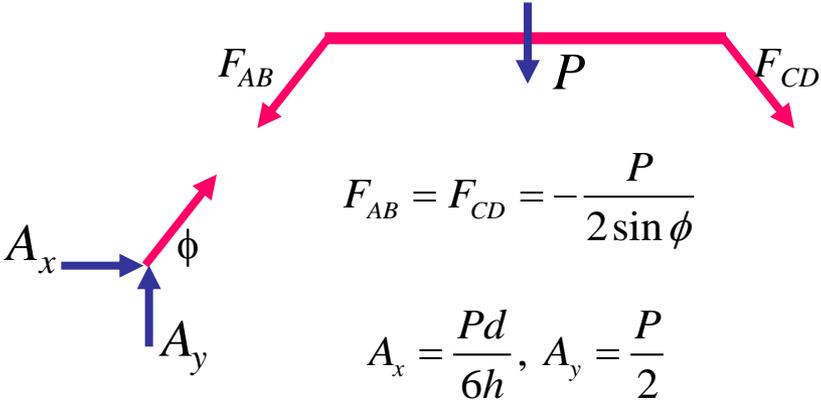
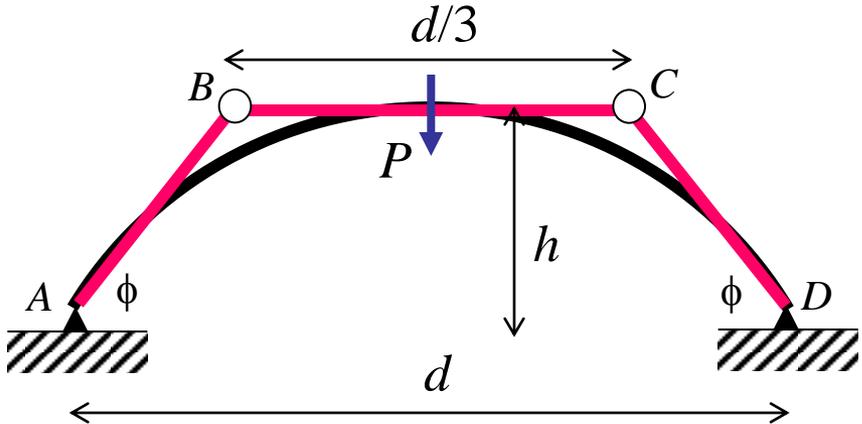
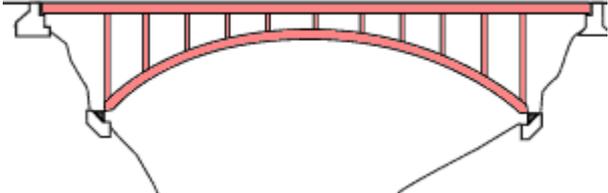
Moments



$$M(x) = -A_x y(x)$$



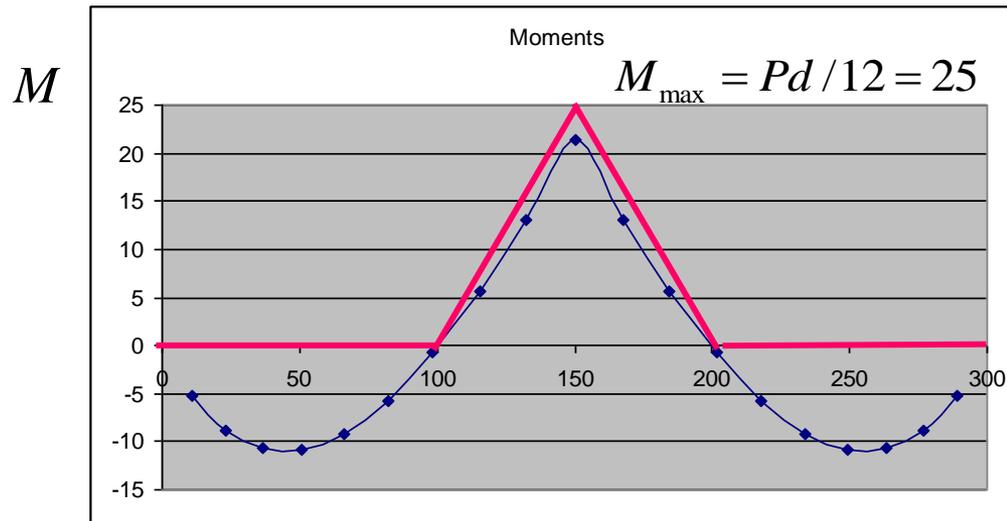
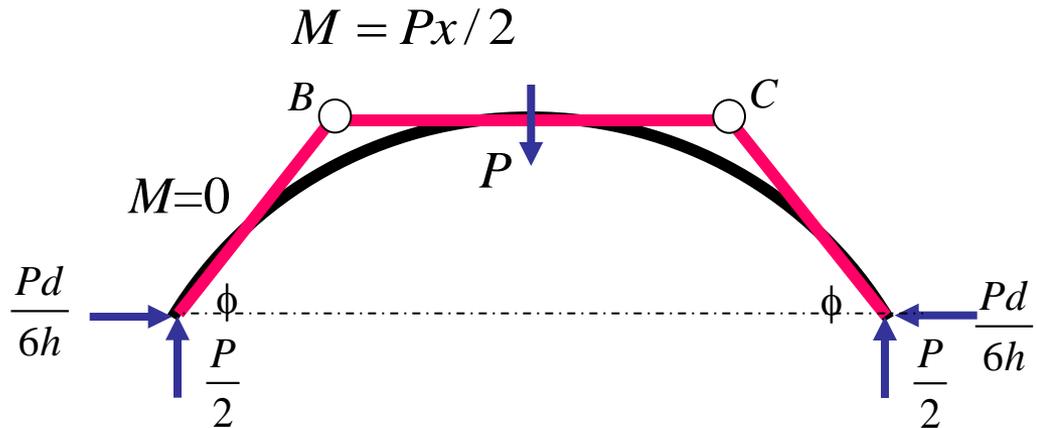
Approximate Analysis



$$F_{AB} = F_{CD} = -\frac{P}{2 \sin \phi}$$

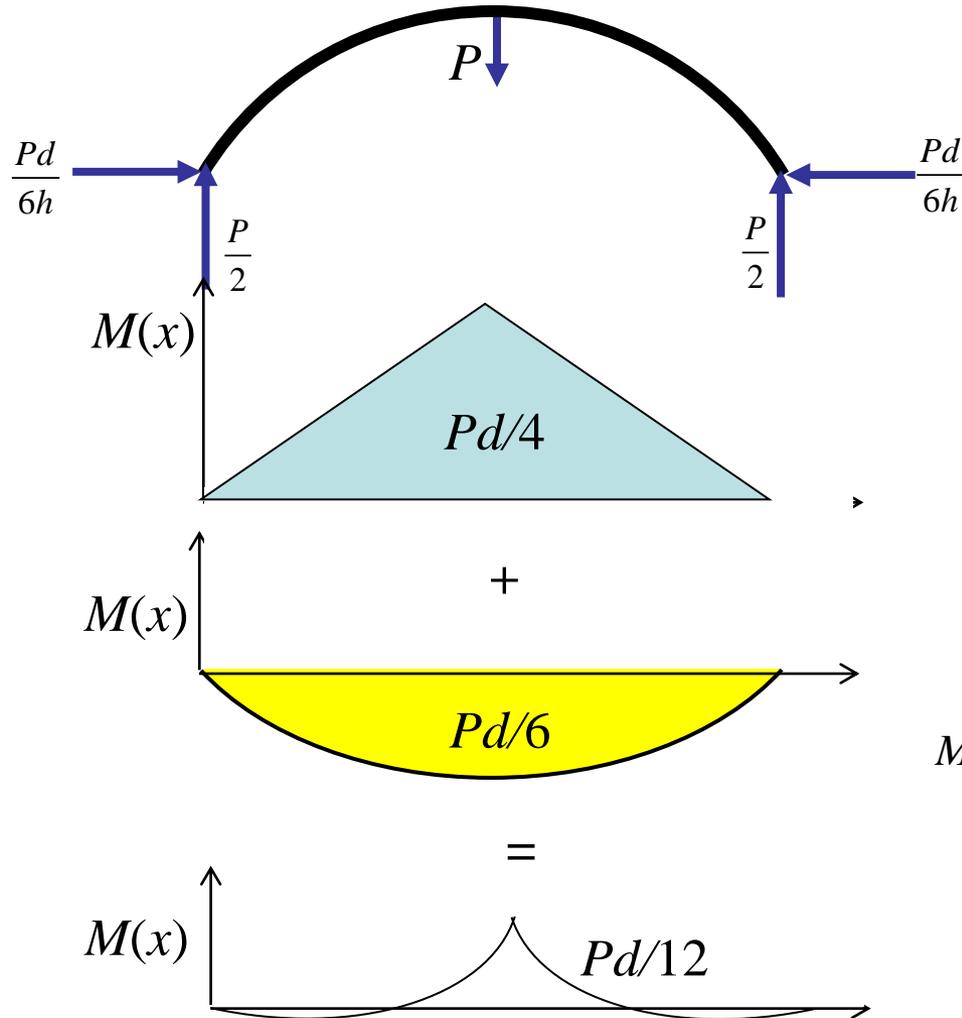
$$A_x = \frac{Pd}{6h}, \quad A_y = \frac{P}{2}$$

Moments (1)

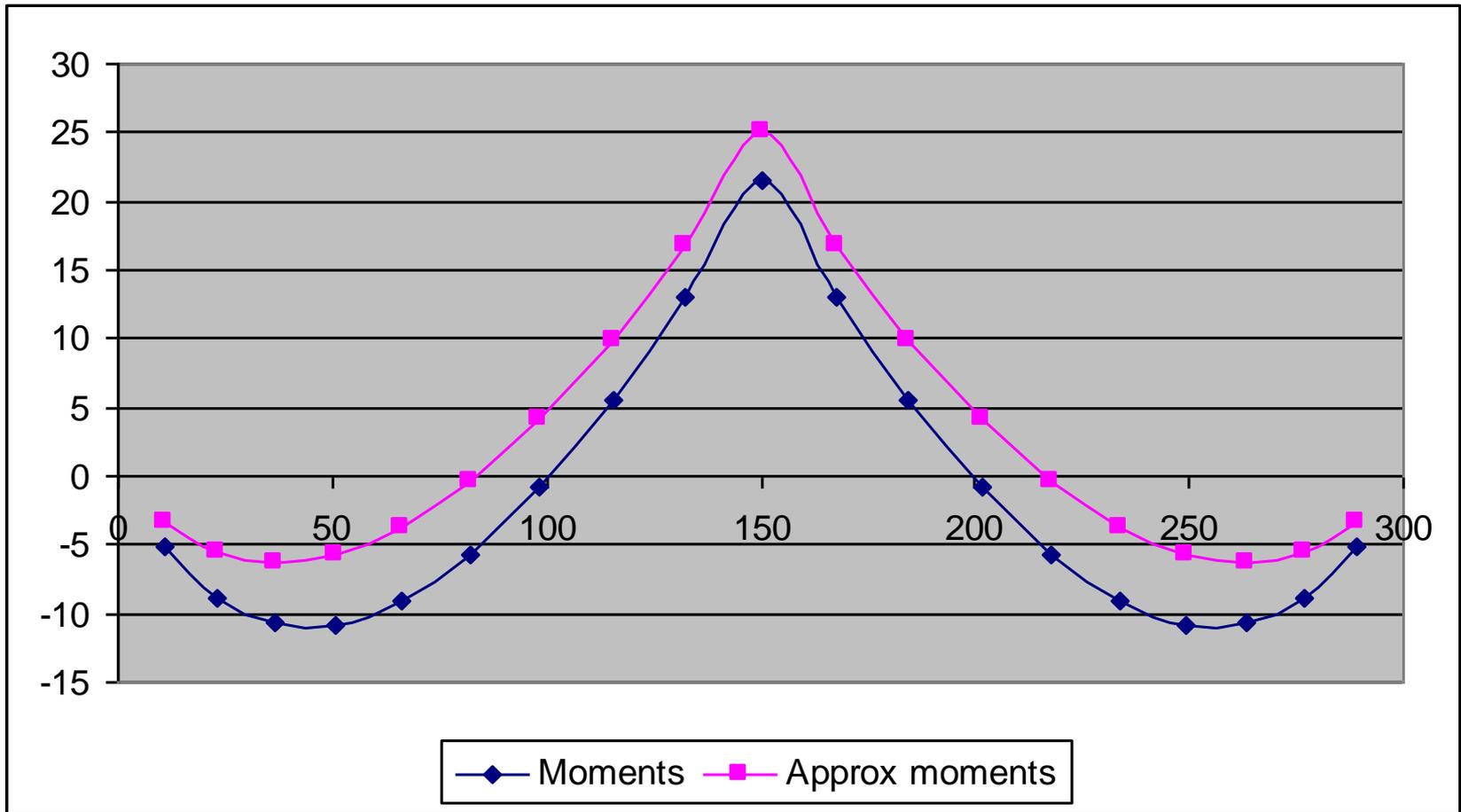


For $E=10^6$, $P=1$, $d=300\text{ft}$, $h=75\text{ft}$, $I=0.0833\text{ft}^4$, $A=1\text{ft}^2$

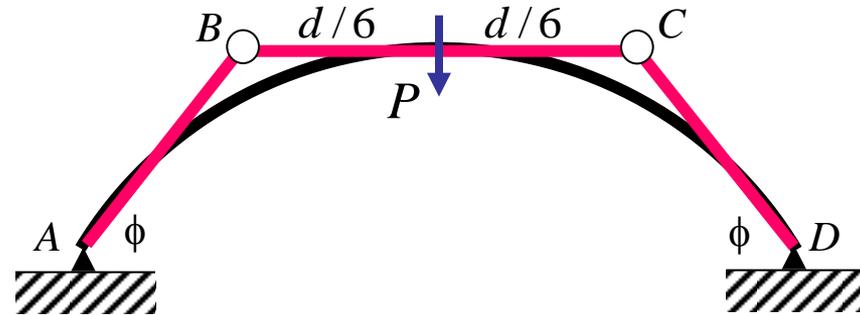
Moments (2)



P=1, d=300ft, h=75ft



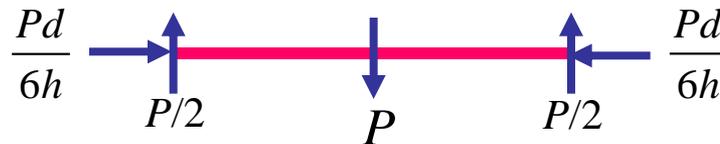
Deflections



$$\tan \phi = \frac{3h}{d},$$

$$L_{AB} = \sqrt{h^2 + d^2/9}$$

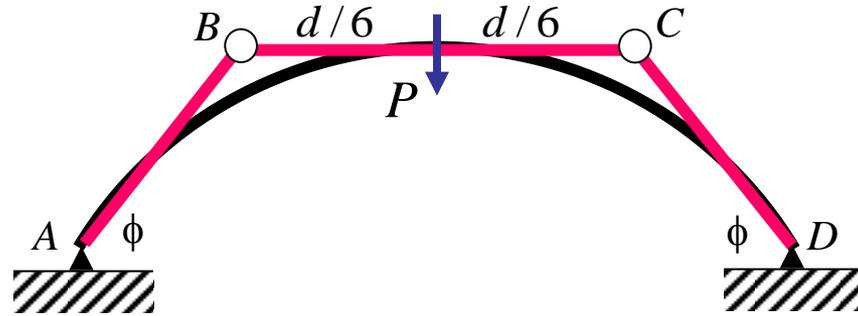
$$\delta_{AB} = \frac{F_{AB} L_{AB}}{EA} = \mathbf{u}_B \cdot \mathbf{n}_{AB} = u_x^B \cos \phi + u_y^B \sin \phi = -\frac{P L_{AB}}{2EA \sin \phi}.$$



$$u_x^B = -\frac{Pd}{6h} \frac{d}{6EA} = \frac{Pd^2}{36hEA}$$

$$u_y^P = u_y^B - \frac{Pd/3}{48EI}$$

Deflections



$$\tan \phi = \frac{3h}{d}$$

$$\sin \phi = \frac{h}{L_{AB}}$$

$$L_{AB} = \sqrt{h^2 + d^2 / 9}$$

$$\frac{Pd^2}{36hEA} \cos \phi + u_y^B \sin \phi = -\frac{PL_{AB}}{2EA \sin \phi}$$

$$\Rightarrow u_y^B = -\frac{PL_{AB}}{2EA \sin^2 \phi} - \frac{Pd^2}{36EAh \tan \phi} = -\frac{P(h^2 + d^2 / 9)^{3/2}}{2EAh^2} - \frac{Pd^3}{108EAh^2}$$

$$u_y^P = u_y^B - \frac{P(d/3)^3}{48EI}$$

For $E=10^6$, $P=1$, $d=300\text{ft}$, $h=75\text{ft}$, $I=0.0833\text{ft}^4$, $A=1\text{ft}^2$

$$u_y^P = -0.25\text{ft}$$

Excel gives 0.28 feet