

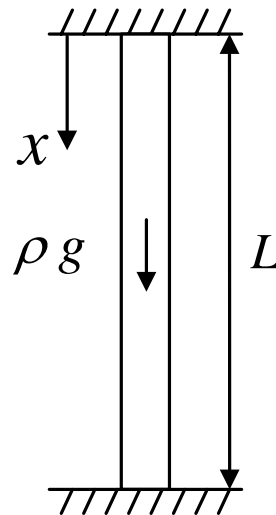
### Principle of minimum potential energy (continued)

The potential energy of a system is

$$V = \int_V w dV - \int_V f_i u_i dV - \int_S t_i u_i dS$$

Principle of minimum potential energy states that for all kinematically admissible  $u_i$ , the actual displacement field minimizes  $V$ .

Example 1:



We have shown in the beginning of the semester that the exact solution is:  $u = \frac{\rho g}{2E} x(L-x)$ .

Now we discuss how to solve the same problem by using the principle of minimum potential energy.

Procedure:

- 1) Pick any displacement such that  $u(0) = u(L) = 0$ .
- 2) Minimize  $V$  for the chosen parameters.

An obvious choice is  $u(x) = x(L-x)f(x)$  since this satisfies the clamped displacement boundary conditions for any  $f(x)$ . We can assume  $f(x)$  to be a polynomial function.

$$f(x) = C_0 + C_1 x + C_2 x^2 + \dots + C_N x^N$$

To minimize the potential energy  $V(C_0, C_1, C_2, \dots, C_N)$ , we take

$$\frac{\partial V}{\partial C_0} = 0, \quad \frac{\partial V}{\partial C_1} = 0, \quad \dots, \quad \frac{\partial V}{\partial C_N} = 0$$

This gives  $N + 1$  algebraic equations to determine  $N + 1$  parameters  $C_0, C_1, C_2, \dots, C_N$ .

For the present example, take the simplest form that  $f(x) = C_0$ .

$$u(x) = C_0 x(L - x)$$

$$\varepsilon = \frac{\partial u}{\partial x} = C_0(L - 2x)$$

$$\sigma = E\varepsilon = EC_0(L - 2x)$$

$$w = \frac{1}{2}\sigma\varepsilon = \frac{1}{2}EC_0^2(L - 2x)^2$$

The potential energy of the system is

$$\begin{aligned} V &= \int_0^L w dx \cdot A - \int_0^L \rho g C_0 x(L - x) dx \cdot A \\ &= \int_0^L \frac{1}{2} EC_0^2 (L - 2x)^2 dx \cdot A - \int_0^L \rho g C_0 x(L - x) dx \cdot A \\ &= \frac{1}{2} A \frac{L^3}{6} (EC_0^2 - \rho g C_0) \end{aligned}$$

To minimize  $V$ ,

$$\frac{\partial V}{\partial C_0} = 0 \Rightarrow 2EC_0 - \rho g = 0 \Rightarrow C_0 = \frac{\rho g}{2E}$$

Therefore  $u = \frac{\rho g}{2E} x(L - x)$ , which is identical to the exact solution for this problem.

Suppose  $f(x) = C_0 + C_1 x$  is assumed, we will get a function of potential energy in terms of

$C_0$  and  $C_1$ ,

$$V(C_0, C_1)$$

Minimizing the potential energy requires that

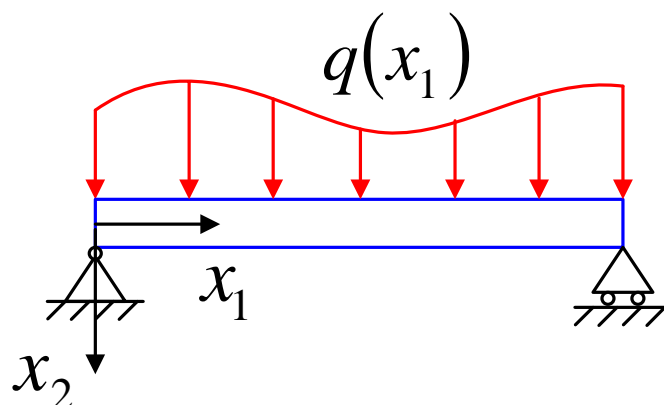
$$\frac{\partial V}{\partial C_0} = 0, \quad \frac{\partial V}{\partial C_1} = 0$$

The parameters turned out to be

$$C_0 = \frac{\rho g}{2E}, \quad C_1 = 0$$

Again we get the exact solution.

Re-derivation of the theory of beam bending



Kinematic assumptions (Kirchhoff):

$$u_2(x_1, x_2) = w(x_1)$$

$$\varepsilon_{12} = \frac{1}{2} \left( \frac{\partial u_1}{\partial x_2} + \frac{\partial u_2}{\partial x_1} \right) = 0$$

$$\frac{\partial \varepsilon_{12}}{\partial x_1} = 0 \Rightarrow \frac{\partial}{\partial x_2} \left( \frac{\partial u_1}{\partial x_1} \right) + w''(x_1) = 0$$

$$\frac{\partial \varepsilon_{11}}{\partial x_2} = \frac{\partial}{\partial x_2} \left( \frac{\partial u_1}{\partial x_1} \right) = -w''(x_1) = -\kappa, \quad \varepsilon_{11} = 0 \quad @ \quad x_2 = 0 \quad (\text{neutral plane})$$

The axial strain is integrated to be

$$\varepsilon_{11} = -\kappa x_2 = -\kappa y$$

The axial stress is  $\sigma_{11} = E\varepsilon_{11}$ . All other stress components are zero. Therefore the strain energy density can be easily calculated

$$w = \frac{1}{2} \sigma_{11} \varepsilon_{11} = \frac{1}{2} E \varepsilon_{11}^2 = \frac{1}{2} E y^2 w''^2$$

Then the potential energy of the system is

$$\begin{aligned} V &= \int_0^L dx \int_A \frac{1}{2} E y^2 w''^2 dA - \int_0^L q(x) w(x) dx \\ &= \int_0^L \frac{1}{2} E I w''^2 dx - \int_0^L q w dx \end{aligned}$$

where  $\int_A y^2 dA = I$  is the moment of inertia.

Recall that the governing equation of beam bending is  $E I w'''' - q = 0$  plus boundary conditions.

Let's see if we get the same equation by using the principle of minimum potential energy.

Since  $V$  is minimum,

$$\delta V = \int_0^L EI w'' \delta w'' dx - \int_0^L q \delta w dx = 0$$

The first term can be integrated by parts

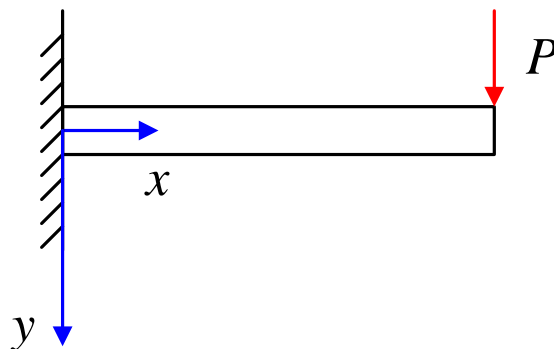
$$\begin{aligned} \int_0^L EI w'' \delta w'' dx &= EI w'' \delta w' \Big|_0^L - \int_0^L EI w''' \delta w' dx \\ &= EI w'' \delta w' \Big|_0^L - EI w''' \delta w \Big|_0^L + \int_0^L EI w'''' \delta w dx \end{aligned}$$

The second terms vanish due to the displacement boundary conditions at the ends, therefore

$$\delta V = EI w'' \delta w' \Big|_0^L + \int_0^L (EI w'''' - q) \delta w dx = 0 \Rightarrow EI w'''' - q = 0 \text{ and } w'' = 0 \text{ @ } x = 0, L$$

which is exactly the governing equation and moment-free boundary conditions for the simply supported beam.

Example 2:



The potential energy of the system is

$$V = \int_0^L \frac{1}{2} EI w''^2 dx - Pw(L)$$

Pick  $w(x)$  that satisfies  $w(0) = 0$ ,  $w'(0) = 0$ , a simple guess is

$$w(x) = Cx^2$$

$$w'' = 2C$$

$$V = \frac{1}{2} EI \int_0^L (2C)^2 dx - PCL^2 = 2C^2 EIL - PCL^2$$

Minimizing potential energy requires

$$\frac{\partial V}{\partial C} = 0 \Rightarrow 4CEIL - PL^2 = 0 \Rightarrow C = \frac{PL}{4EI}$$

Let us compare the resulted solution to exact one,

$$w(L) = \frac{PL^3}{4EI} \quad (\text{Exact solution: } w(L) = \frac{PL^3}{3EI})$$

The relative error is:  $\frac{1/3 - 1/4}{1/3} = 25\%$

The above solution can be improved by taking more terms.

Suppose  $w(x) = C_1 x^2 + C_2 x^3$ . The system potential energy becomes

$$V = \frac{1}{2} EI \int_0^L (2C_1 + 6C_2 x)^2 dx - P(C_1 L^2 + C_2 L^3)$$

The same conditions that minimize potential energy

$$\frac{\partial V}{\partial C_1} = 0, \quad \frac{\partial V}{\partial C_2} = 0$$

The parameters are determined as

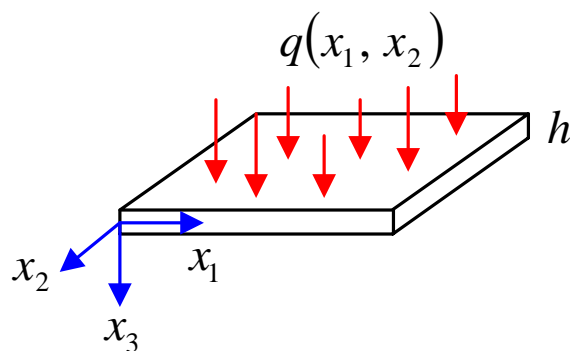
$$C_1 = -\frac{P}{2EI} L, \quad C_2 = \frac{P}{6EI}$$

Therefore:

$$w(L) = C_1 L^2 + C_2 L^3 = \frac{PL^3}{3EI}$$

which is the exact solution.

Extend the analysis to plate bending



Kinematic assumption (Kirchhoff):

$$u_3(x_1, x_2, x_3) = w(x_1, x_2)$$

$$\varepsilon_{13} = \varepsilon_{23} = 0$$

$$\frac{\partial u_1}{\partial x_3} + \frac{\partial u_3}{\partial x_1} = 0 \quad (1)$$

$$\frac{\partial u_2}{\partial x_3} + \frac{\partial u_3}{\partial x_2} = 0 \quad (2)$$

$$\frac{\partial(1)}{\partial x_1} \Rightarrow \frac{\partial \varepsilon_{11}}{\partial x_3} = -\frac{\partial^2 w}{\partial x_1^2}$$

$$\frac{\partial(2)}{\partial x_2} \Rightarrow \frac{\partial \varepsilon_{22}}{\partial x_3} = -\frac{\partial^2 w}{\partial x_2^2}$$

$$\frac{1}{2} \left( \frac{\partial(1)}{\partial x_2} + \frac{\partial(2)}{\partial x_1} \right) \Rightarrow \frac{\partial \varepsilon_{12}}{\partial x_3} = -\frac{\partial^2 w}{\partial x_1 \partial x_2}$$

Assume @  $x_3 = 0$ ,  $\varepsilon_{11} = \varepsilon_{22} = \varepsilon_{12} = 0$  (neutral plane), the strain components can be integrated out, respectively

$$\varepsilon_{11} = -\frac{\partial^2 w}{\partial x_1^2} x_3, \quad \varepsilon_{22} = -\frac{\partial^2 w}{\partial x_2^2} x_3, \quad \varepsilon_{12} = -\frac{\partial^2 w}{\partial x_1 \partial x_2} x_3$$

The strain energy density is

$$\begin{aligned} w &= \frac{1}{2} \sigma_{11} \varepsilon_{11} + \frac{1}{2} \sigma_{22} \varepsilon_{22} + \frac{1}{2} (\sigma_{12} \varepsilon_{12} + \sigma_{21} \varepsilon_{21}) \\ &= \frac{E}{2(1-\nu^2)} (\varepsilon_{11}^2 + \varepsilon_{22}^2 + 2\nu \varepsilon_{11} \varepsilon_{22} + 2(1-\nu) \varepsilon_{12}^2) \\ &= \frac{E}{2(1-\nu^2)} (w_{,11}^2 + w_{,22}^2 + 2\nu w_{,11} w_{,22} + 2(1-\nu) w_{,12}^2) x_3^2 \end{aligned}$$

The system potential energy is

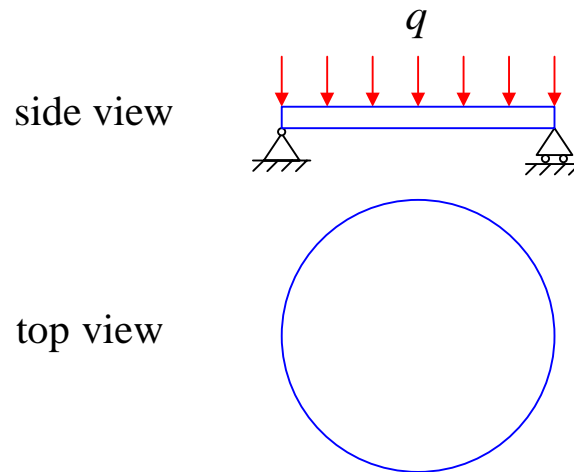
$$\begin{aligned} V &= \int_V w dV - \int_A q w dA \\ &= \frac{Eh^3}{24(1-\nu^2)} \int_A (w_{,11}^2 + w_{,22}^2 + 2\nu w_{,11} w_{,22} + 2(1-\nu) w_{,12}^2) dA - \int_A q w dA \end{aligned}$$

Define  $D = \frac{Eh^3}{12(1-\nu^2)}$  and minimize  $V$ . We have

$$\begin{aligned} \delta V &= \int_V w dV - \int_A q w dA \\ &= D \int_A (w_{,11} \delta w_{,11} + w_{,22} \delta w_{,22} + \nu w_{,11} \delta w_{,22} + \nu w_{,22} \delta w_{,11} + 2(1-\nu) w_{,12} \delta w_{,12}) dA - \int_A q \delta w dA \\ &= \int_A (D \nabla^2 \nabla^2 w - q) \delta w dA + \text{boundary terms} = 0 \\ &\Rightarrow D \nabla^2 \nabla^2 w - q = 0 \end{aligned}$$

This is the classical governing equation for plate bending. In the following, we illustrate the application of energy theorem in solving plate bending problems.

Example 3: Simply supported circular plate under uniform pressure



Displacement boundary condition  $w|_{r=a} = 0$ .

To satisfy the boundary condition, pick the kinematically admissible displacement  $w = C(r^2 - a^2) = C(x^2 + y^2 - a^2)$ . With this assumption,

$$w_{,11} = w_{,22} = 2C, \quad w_{,12} = 0$$

The system potential energy can be easily integrated as

$$\begin{aligned} V &= \frac{D}{2} \int_A \left( (2C)^2 + (2C)^2 + 2\nu(2C)^2 \right) dA - \int_A qC(r^2 - a^2) dA \\ &= 4D(1+\nu)C^2 \cdot \pi a^2 + \frac{\pi}{2} a^4 qC \end{aligned}$$

To minimize potential energy,

$$\frac{\partial V}{\partial C} = 0 \Rightarrow 8D(1+\nu)C \cdot \pi a^2 + \frac{\pi}{2} a^4 q \Rightarrow C = -\frac{qa^2}{16D(1+\nu)}$$

The deflection of the plate is

$$w = -\frac{qa^2}{16D(1+\nu)}(r^2 - a^2)$$

The maximum deflection occurs at the center,

$$w_{\max} = w|_{r=0} = \frac{qa^4}{16D(1+\nu)}$$

The exact solution to this problem shows:  $w|_{r=0} = \frac{qa^4}{64D} \frac{5+\nu}{1+\nu}$ . The relative error is

$$\frac{(5+\nu)/64 - 1/16}{(5+\nu)/64} = \frac{1+\nu}{5+\nu} \approx 25\%.$$

We see that a simple guess leads to a solution fairly close to exact.