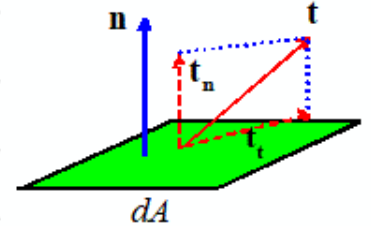


5 Describing Internal Forces in Solids

5.1 External Forces

Traction Vector $\underline{t} = \lim_{dA \rightarrow 0} \frac{dP}{dA}$

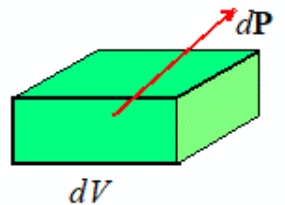


Normal traction $t_n = \underline{t} \cdot \underline{n}$

Tangential $\underline{t}_t = \underline{t} - t_n \underline{n}$

Total force on surface : $\underline{P} = \int_S \underline{t} dA$

Body force vector (force per unit mass)



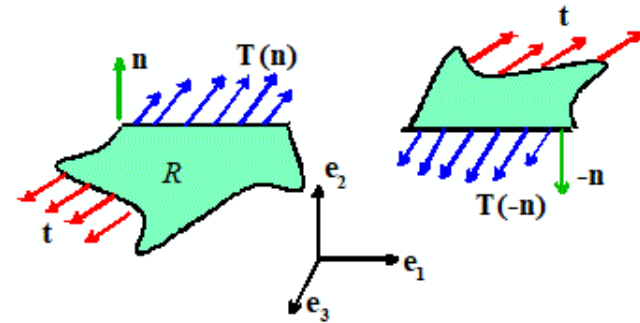
$$\underline{b} = \lim_{dV \rightarrow 0} \frac{dP}{\rho dV}$$

ρ : mass density

Resultant body force $\underline{P} = \int_V \rho \underline{b} dV$

5.2 Internal traction vector

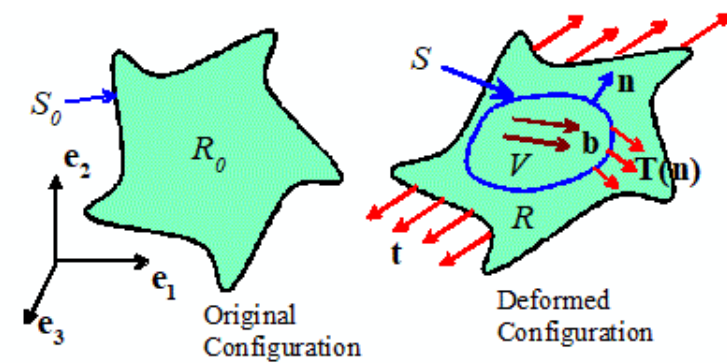
Internal traction $\underline{T}(\underline{n}) = \lim_{dA \rightarrow 0} \frac{d\underline{P}(\underline{n})}{dA}$



Total force acting on a sub-volume of solid

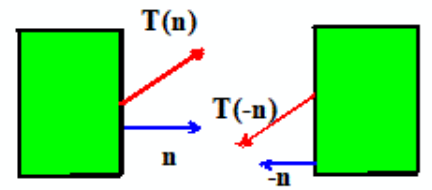
$$\underline{P} = \int_S \underline{T}(\underline{n}) dA + \int_V \rho \underline{b} dV$$

\underline{n} : unit vector



Properties of $\underline{T}(\underline{n})$

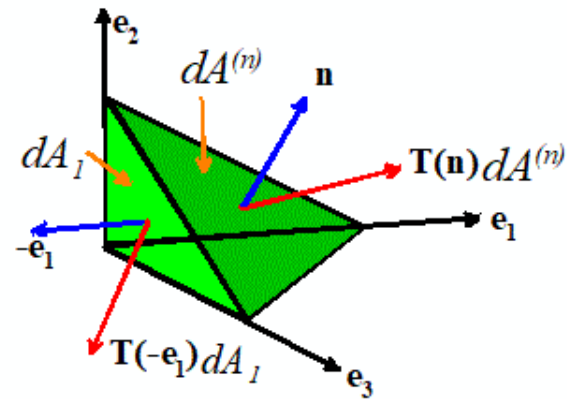
- ① Traction on adjacent planes are equal & opposite



$$\underline{T}(-\underline{n}) = -\underline{T}(\underline{n})$$

- ② Traction on surfaces passing through a point are related

$$\underline{T}(\underline{n}) = \underline{T}(\underline{e}_1)n_1 + \underline{T}(\underline{e}_2)n_2 + \underline{T}(\underline{e}_3)n_3$$



(Consequence of $\underline{F} = m\underline{a}$)

$$\underline{n} = n_1\underline{e}_1 + n_2\underline{e}_2 + n_3\underline{e}_3$$

Proof

① Show first that

$$dA_n \underline{n} = dA_1 \underline{e}_1 + dA_2 \underline{e}_2 + dA_3 \underline{e}_3$$

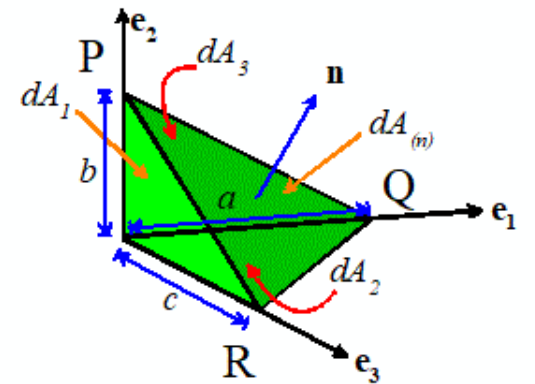
Note $2dA_n \underline{n} = \vec{PR} \times \vec{PQ}$

$$= (c\underline{e}_3 - b\underline{e}_2) \times (a\underline{e}_1 - b\underline{e}_2)$$

$$= cb\underline{e}_3 + ca\underline{e}_2 + ab\underline{e}_3$$

$$= 2(dA_1 \underline{e}_1 + dA_2 \underline{e}_2 + dA_3 \underline{e}_3) \quad \text{QED!}$$

Hence $\underline{n} = \underbrace{\frac{dA_1}{dA_n}}_{n_1} \underline{e}_1 + \underbrace{\frac{dA_2}{dA_n}}_{n_2} \underline{e}_2 + \underbrace{\frac{dA_3}{dA_n}}_{n_3} \underline{e}_3$



② Now $\underline{F} = m\underline{a}$ for tetrahedron

$$\underline{T}(\underline{n}) dA_n + \underline{T}(-\underline{e}_1) dA_1 + \underline{T}(-\underline{e}_2) dA_2 + \underline{T}(-\underline{e}_3) dA_3 + \underline{\rho} b dV = \underline{\rho} dV \underline{a}$$

Divide through by dA_n ; note $\lim_{dV \rightarrow 0} \frac{dV}{dA_n} \rightarrow 0$

$$\underline{T}(\underline{n}) - \underline{T}(\underline{e}_1) \underbrace{\frac{dA_1}{dA_n}}_{n_1} - \underline{T}(\underline{e}_2) \underbrace{\frac{dA_2}{dA_n}}_{n_2} - \underline{T}(\underline{e}_3) \underbrace{\frac{dA_3}{dA_n}}_{n_3} = 0$$

Q.E.D !

5.3 Cauchy Stress Tensor

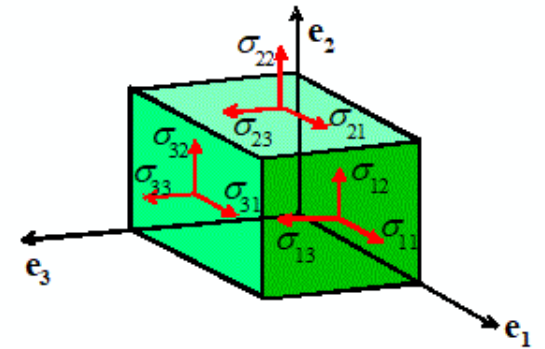
Define $\sigma_{ij} = T_j(\underline{e}_i)$ (9 numbers)

Then $T_j(\underline{n}) = n_i \sigma_{ij}$ $\underline{T}(\underline{n}) = \underline{n} \sigma$

Note σ maps \underline{n} onto \underline{T} \Rightarrow tensor!

Physical Significance

σ_{ij} = force per unit deformed area acting in j direction on plane with normal in i direction

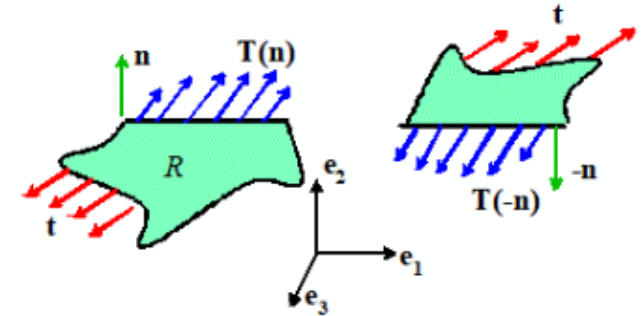


Health warning: some people use transpose $\underline{T} = \sigma \underline{n}$
 - fortunately $\sigma_{ij} = \sigma_{ji}$ (symmetric)

Other stress measures

Cauchy stress is 'True Stress' (Force per unit deformed area)

$$T_j(\mathbf{n}) = n_i \sigma_{ij} \quad \mathbf{T} = \mathbf{n} \cdot \boldsymbol{\sigma} \quad \text{True stress is symmetric}$$

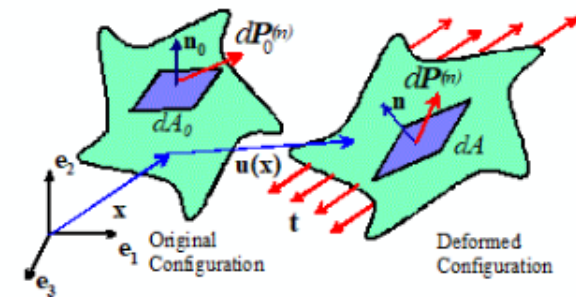


There are also several types of 'Nominal Stress' (Force per unit undeformed area)

Deformation gradient $\mathbf{F} = \mathbf{I} + \nabla \mathbf{u} \quad F_{ij} = \delta_{ij} + \frac{\partial u_i}{\partial x_j}$

Jacobian $J = \det(\mathbf{F})$

Kirchhoff stress $\boldsymbol{\tau} = J\boldsymbol{\sigma}$



Nominal (First Piola-Kirchhoff Stress) $\mathbf{S} = J\mathbf{F}^{-1} \cdot \boldsymbol{\sigma} \quad S_{ij} = JF_{ik}^{-1} \sigma_{kj}$

Has property that $d\mathbf{P} = dA_0 \mathbf{n}_0 \cdot \mathbf{S} = dA \mathbf{n} \cdot \boldsymbol{\sigma}$ Unsymmetric – some texts use transpose

Material (Second Piola-Kirchhoff Stress) $\boldsymbol{\Sigma} = J\mathbf{F}^{-1} \cdot \boldsymbol{\sigma} \cdot \mathbf{F}^{-T} \quad \Sigma_{ij} = JF_{ik}^{-1} \sigma_{kl} F_{jl}^{-1}$

Has property that $d\mathbf{P}_0 = \mathbf{F}^{-1} d\mathbf{P} = dA_0 \mathbf{n}_0 \cdot \boldsymbol{\Sigma}$ (Weird but useful for material models)

Example: Consider a hydrostatic stress $\sigma_{ij} = -p\delta_{ij}$

Show that the traction acts normal to any internal plane in the solid (fluid?) and find its magnitude

$$\sigma = \begin{bmatrix} -p & 0 & 0 \\ 0 & -p & 0 \\ 0 & 0 & -p \end{bmatrix}$$

Formula $T_i(n_j) = n_j \sigma_{ji} = n_j (-p\delta_{ij})$

$$T_i(n_j) = -p n_i$$

$$\underline{T}(n) = -p \underline{n}$$

Direction of \underline{T} is parallel to \underline{n}

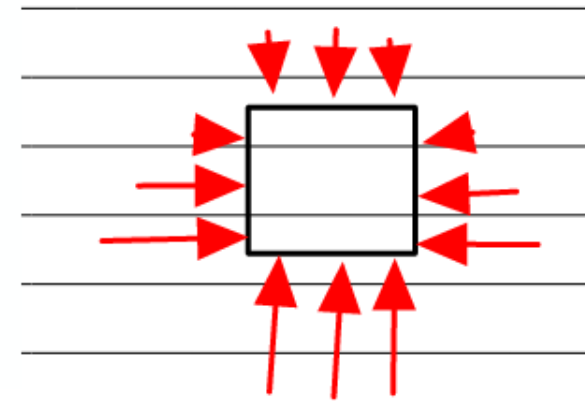
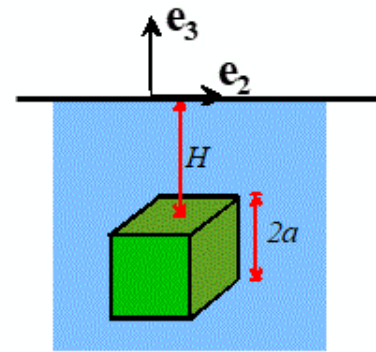
mag ↑
Dir ↓

(hydrostatic pressure)

Example: A cube is immersed in fluid with stress tensor

$$\sigma_{ij} = \rho g x_3 \delta_{ij}$$

Find the resultant force on the cube



Forces on sides with normals $\underline{e}_1, \underline{e}_2$ are equal & opposite - no horizontal force

Force on top face $\rho g H (2a)^2 (-\underline{e}_3)$

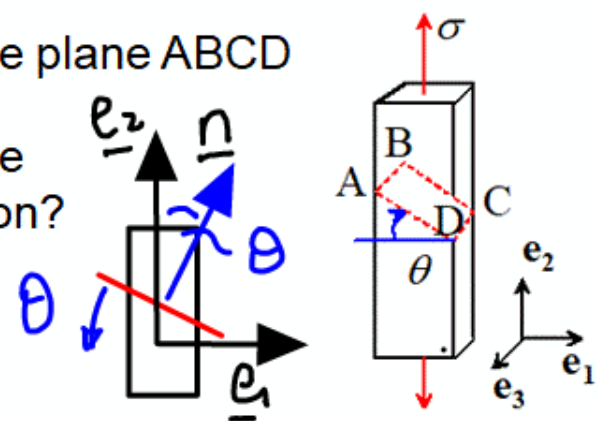
on bottom face $\rho g (H+2a) (2a)^2 (\underline{e}_3)$

Total : $\underline{P} = \underbrace{(2a)^3 \rho g}_{\text{weight of displaced fluid}} \underline{e}_3$

weight of displaced fluid

Example: Find the tractions on the plane ABCD

What is the inclination of the plane with the greatest tangential traction?



Stress tensor

$$\sigma = \begin{bmatrix} 0 & 0 & 0 \\ 0 & \sigma & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

Formula $\underline{T} = \underline{n} \sigma$ Here $\underline{n} = [\sin\theta \quad \cos\theta \quad 0]$

$$\underline{T} = \begin{bmatrix} \sin\theta & \cos\theta & 0 \end{bmatrix} \begin{bmatrix} 0 & 0 & 0 \\ 0 & \sigma & 0 \\ 0 & 0 & 0 \end{bmatrix} = \begin{bmatrix} 0 \\ \sigma \cos\theta \\ 0 \end{bmatrix}$$

Normal traction $\underline{T}_n = \underline{T} \cdot \underline{n} = \sigma \cos^2\theta$

Tangential $\underline{T}_t = \underline{T} - \underline{T}_n \underline{n} = \begin{bmatrix} 0 \\ \sigma \cos\theta \\ 0 \end{bmatrix} - \sigma \cos^2\theta \begin{bmatrix} \sin\theta \\ \cos\theta \\ 0 \end{bmatrix}$

$$|\underline{T}_t| = \sigma \cos\theta \sqrt{(\sin\theta \cos\theta)^2 + (1 - \cos^2\theta)^2} = \sigma \sin\theta \cos\theta$$

$$= \frac{1}{2} \sigma \sin 2\theta \quad \max\{|\underline{T}_t|\} = \sigma/2 \text{ for } \theta = \pi/4$$

5.4 Principal Stresses

Note σ is symmetric

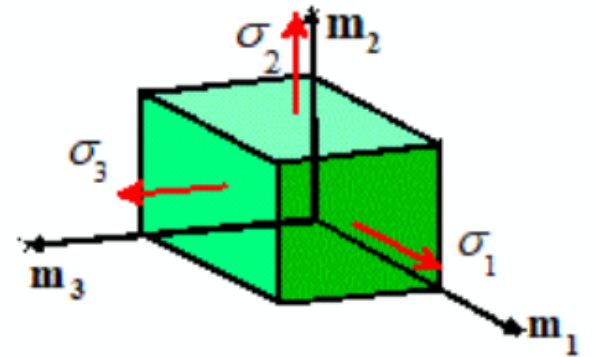
We can find a special basis in which σ is diagonal

$$[\sigma^{(m)}] = \begin{bmatrix} \sigma_1 & 0 & 0 \\ 0 & \sigma_2 & 0 \\ 0 & 0 & \sigma_3 \end{bmatrix}$$

σ_i : eigenvalues of $[\sigma]$

$\{ \underline{m}_1, \underline{m}_2, \underline{m}_3 \}$: eigenvectors

Force per unit area on planes perpendicular to \underline{m}_i is normal to surface



Some useful properties of principal stresses

① Maximum normal stress (traction) on any plane @ a point is $\max(\sigma_1, \sigma_2, \sigma_3)$

② max shear stress on any plane @ a point is

$$\tau_{\max} = \frac{1}{2} \max \{ \underbrace{|\sigma_1 - \sigma_2|}, \underbrace{|\sigma_1 - \sigma_3|}, |\sigma_2 - \sigma_3| \}$$

plane @
45° to
($\underline{e}_1, \underline{e}_2$)

plane @
45° to
($\underline{e}_1, \underline{e}_3$)