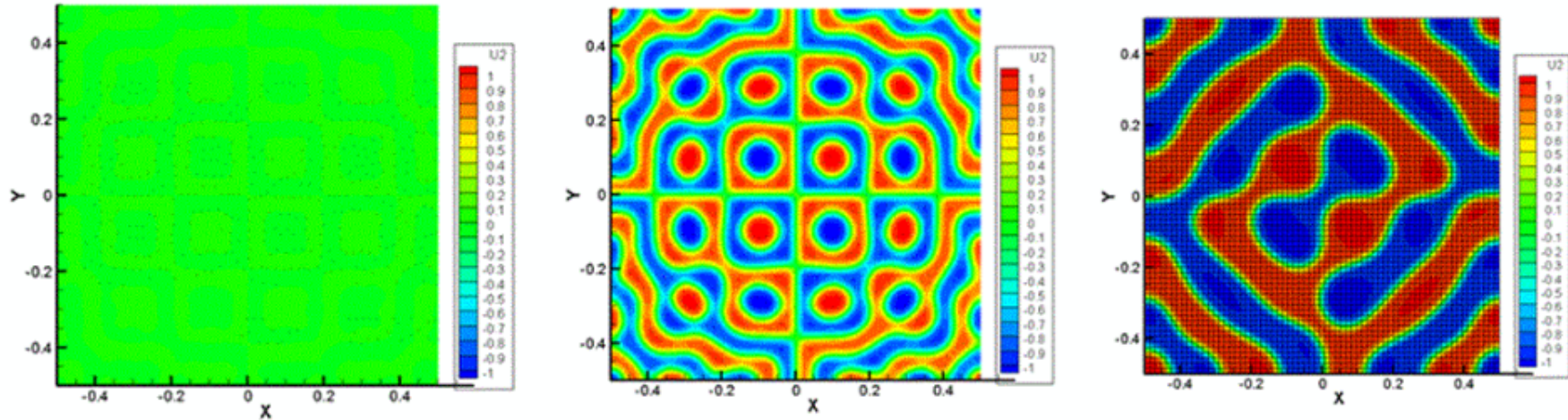


Review – Solving the Cahn-Hilliard Equation



Free energy: $G(c) = \frac{1}{4}(c^2 - 1)^2 + \frac{1}{2}\kappa|\nabla c|^2$

Evolution equation for concentration $\frac{\partial c}{\partial t} = \nabla \cdot D \nabla \mu$ $\mu = \frac{\delta G}{\delta c} = c(c^2 - 1) - \kappa \nabla^2 c$

Discrete weak form $M_{ab} \frac{dc^b}{dt} + DK_{ab} \mu^b = 0$ $M_{ab} \mu^b - F^a(c^b) + \kappa K_{ab} c^b = 0$

$$M_{ab} = \int_V N^a N^b dV \quad K_{ab} = \int_V \frac{\partial N^a}{\partial x_i} \frac{\partial N^b}{\partial x_i} dV \quad F^b = \int_V c(c^2 - 1) N^b dV$$

Time integration

$$M_{ab} (\mu^b + \Delta \mu^b) - F^a(c^b + \Delta c^b) - \kappa K_{ab} (c^b + \Delta c^b) = 0$$

$$M_{ab} \frac{\Delta c^b}{\Delta t} + DK_{ab} [(1 - \theta) \mu^b(t) + \theta (\mu^b(t) + \Delta \mu^b)] = 0$$

Effects of θ :

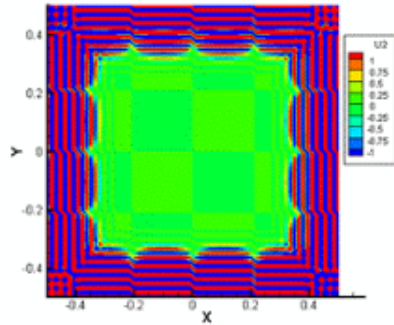
Choosing $\theta = 0$
makes time stepping
unstable

$\theta > 0.5$ is stable

$\theta = 0.5$ gives
best accuracy

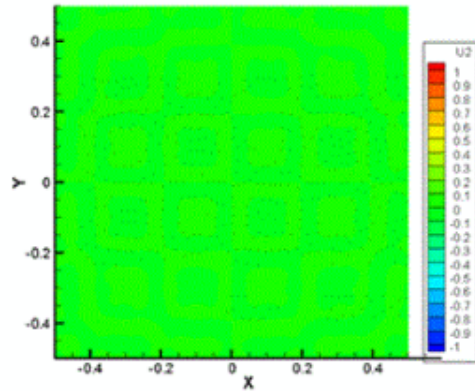
$\theta = 1$ gives best
stability

\Rightarrow Instability problems
sensitive to choice

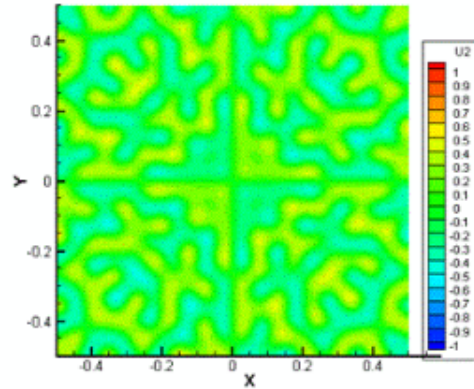
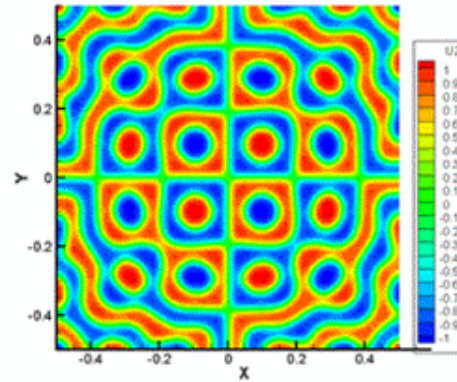


$\theta = 0 \quad \Delta t = 10^{-3} \quad D = 1, \quad \kappa = 0.0001$

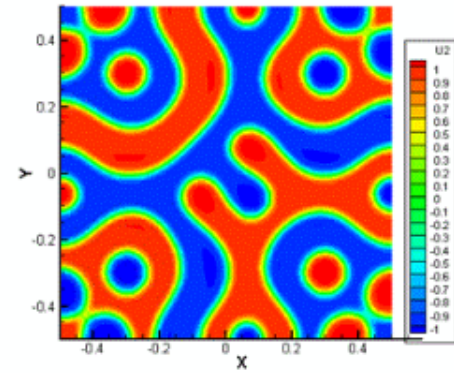
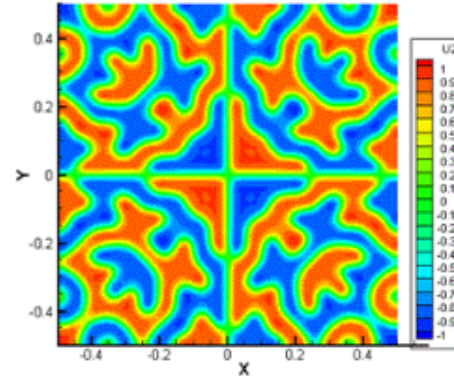
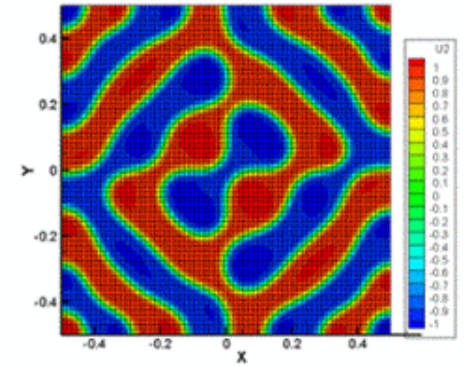
(unstable even for very small timesteps)



$\theta = 0.5 \quad \Delta t = 10^{-3} \quad D = 1, \quad \kappa = 0.0001$



$\theta = 1 \quad \Delta t = 10^{-3} \quad D = 1, \quad \kappa = 0.0001$



8.2 Solving Dynamic problems in solids

Governing Equations

BLM:
$$\frac{d\sigma_{ij}}{dy_j} = \rho \frac{\partial^2 u_i}{\partial t^2} \Big|_{\underline{y} = \text{const}}$$

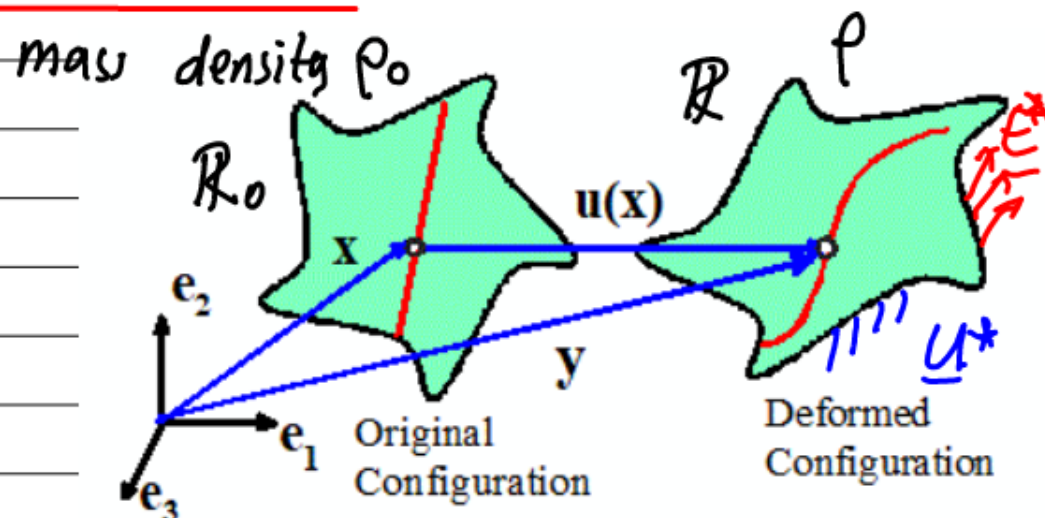
Initial conditions

$$u_i = u_i^0 \quad \frac{\partial u_i}{\partial t} = v_i^0 \quad t=0$$

Boundary conditions $\sigma_{ij} n_j = t_i^*$ on S_2 $u_i = u_i^*$ S_1

Constitutive equation (eg linear elasticity)

$$\sigma_{ij} = C_{ijke} \frac{\partial u_k}{\partial x_e} \quad (\text{other models ok too})$$



$$\rho_0 = J \rho \quad J = \det(F)$$

Weak form of BLM is

$$\int_{R_0} J \sigma_{ij} \frac{\partial \eta_i}{\partial y_j} dV_0 + \int_{R_0} \rho_0 \frac{\partial^2 u_i}{\partial t^2} \eta_i dV_0 = \int_{S_2} t_i^* \eta_i dA_0 \quad \forall \eta_i$$

Discretize: Use usual interpolations $u_i = N^a u_i^a$ $\eta_i = N^a \eta_i^a$

Subst into weak form

$$M_{ab} \ddot{u}_i^b + R_i^a [u_k^b] = F_i^a(t) \quad *$$

$$M_{ab} = \int_{R_0} \rho_0 N^a N^b dV_0 \quad - \text{mass matrix}$$

$$R_i^a = \int_{R_0} J \sigma_{ij} \frac{\partial N^a}{\partial y_j} dV_0 \quad F_i^a = \int_{S_2} t_i^* N^a dA$$

This is all just like statics, except that we need to integrate BLM (*) wrt t

FEA usually uses "Newmark" time-stepping

Two versions : (1) "Implicit" dynamics

(2) "Explicit" dynamics

Also for linear elasticity - "Modal dynamics"

Newmark Time Integration

Given \underline{U}^n , \underline{V}^n , \underline{a}^n at time t_n $\{\underline{U}, \underline{V}, \underline{a}\}$ are FEA vectors

Goal: get \underline{U}^{n+1} , \underline{V}^{n+1} , \underline{a}^{n+1} at $t_{n+1} = t_n + \Delta t$

Simple time-stepping

$$\underline{V}^{n+1} = \underline{V}^n + \Delta t \{ (1-\beta_1) \underline{a}^n + \beta_1 \underline{a}^{n+1} \}$$

$$\underline{U}^{n+1} = \underline{U}^n + \Delta t \underline{V}^n + \frac{\Delta t^2}{2} \left((1-\beta_2) \underline{a}^n + \beta_2 \underline{a}^{n+1} \right)$$

$$0 < \beta_1 < 1 \quad 0 < \beta_2 < 1$$

$$\beta_2 = 0 \text{ — Explicit Dynamics}$$

We find \underline{a}^{n+1} from momentum balance

$$[M] \underline{a}^{n+1} + R \left[\underline{u}^n + \Delta t \underline{v}^n + \frac{\Delta t^2}{2} (1 - \beta_2) \underline{a}^n + \frac{\Delta t^2}{2} \beta_2 \underline{a}^{n+1} \right] = \underline{F}(t + \Delta t)$$

For $\beta_2 \neq 0$ this is a nonlinear system for \underline{a}^{n+1}

Solve with Newton-Raphson

(1) Guess $\underline{a}^{n+1} = \underline{w}$; Correct

$$(2) \left\{ [M] + \frac{\Delta t^2}{2} \beta_2 [K] \right\} d\underline{w} = -[M] \underline{w} - R(\underline{a}^{n+1} = \underline{w}) + \underline{F}(t + \Delta t) \quad **$$

(3) $\underline{w} \rightarrow \underline{w} + d\underline{w}$; check convergence ; repeat

$$[K] = \frac{\partial \underline{R}}{\partial \Delta \underline{u}} \quad (\text{same as static stiffness matrix})$$

(Small change to static equation system)

Newmark algorithm:

Initialize: $\underline{u}^0, \underline{v}^0$ - given $\underline{a}^0 = [M]^{-1} (\underline{F} - R(\underline{u}^0))$

Time step: (general)

(1) Solve $**$ for \underline{a}^{n+1} (Newton-Raphson)

(2) $\underline{v}^{n+1} = \underline{v}^n + \Delta t ((1-\beta_1) \underline{a}^n + \beta_1 \underline{a}^{n+1})$

(3) $\underline{u}^{n+1} = \underline{u}^n + \Delta t \underline{v}^n + \frac{\Delta t^2}{2} ((1-\beta_2) \underline{a}^n + \beta_2 \underline{a}^{n+1})$

For $\beta_2 = 0$ $\underline{a}^{n+1} = [M]^{-1} (\underline{F} - R(\underline{u}^{n+1}))$

("Explicit" update)

\uparrow only depends on \underline{a}^n

Mass Matrices

$$m_{ab} = \int_{\mathcal{R}_0} \rho_0 N^a N^b dV_0$$

- Compute using usual quadrature - 1 order higher integration

"Consistent" mass matrix

"Lumped" mass matrix - for explicit dynamics we replace m_{ab} with a diagonal approximation so $[M]^{-1}$ is easy

Lumping schemes:

(1) "Row Sum" $\tilde{m}_{aa} = \sum_b m_{ab}$ $m_{ab} = 0$ $a \neq b$

(2) "Diagonal Scaling" $\tilde{m}_{aa} = c m_{aa}$ $c = \frac{\sum_a \tilde{m}_{aa}}{\sum_a \sum_b m_{ab}}$

(3) Compute m_{ab} with int pts @ nodes

Lumped mass matrices for 2D elements

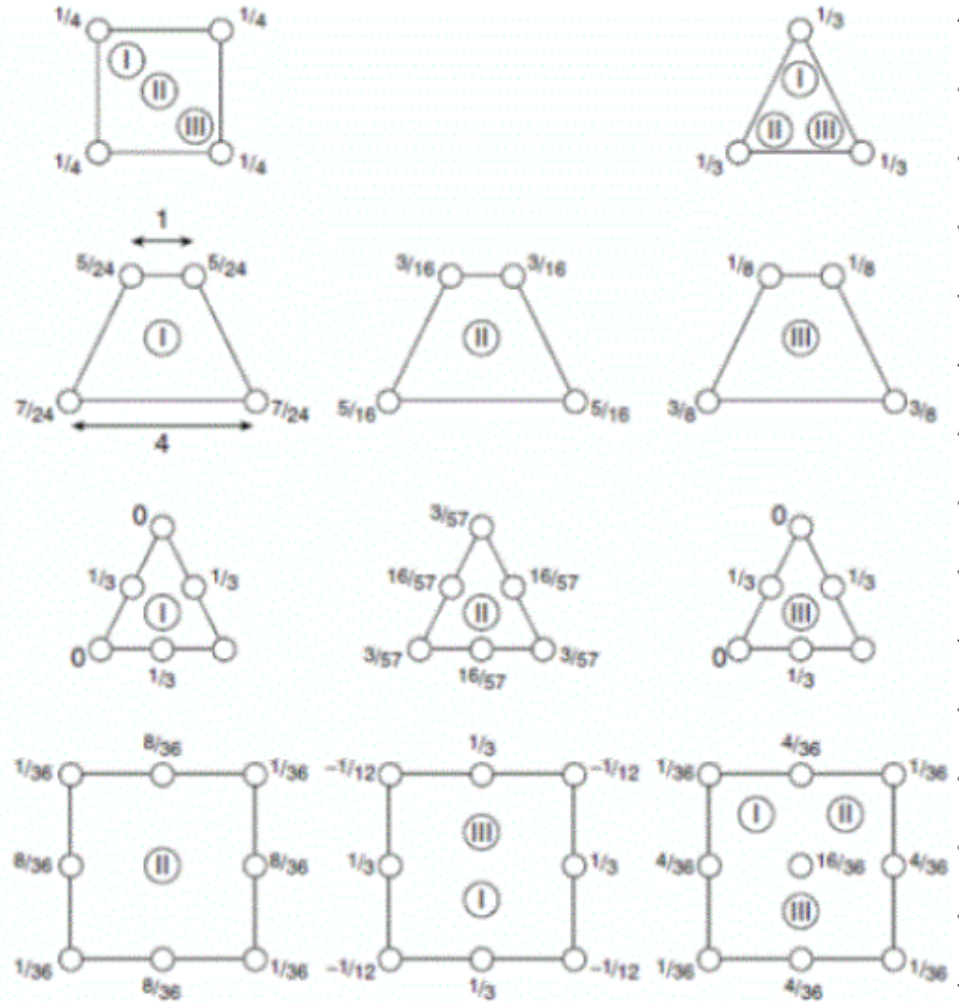
eg for linear quad

$$[M]^{el} = \text{mass of el} \begin{bmatrix} 1/4 & 1/4 \\ 1/4 & 1/4 \end{bmatrix}$$

Different methods give different answers

- Choose the most sensible one!

- Linear elements generally preferable in explicit.



- (I) Row sum procedure
- (II) Diagonal scaling procedure
- (III) Quadrature using nodal points