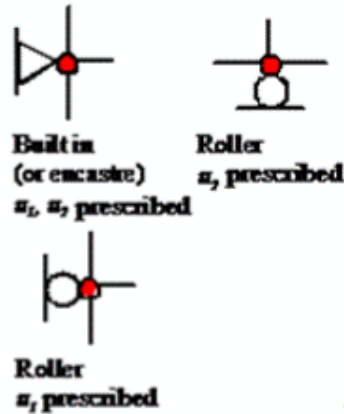


Review

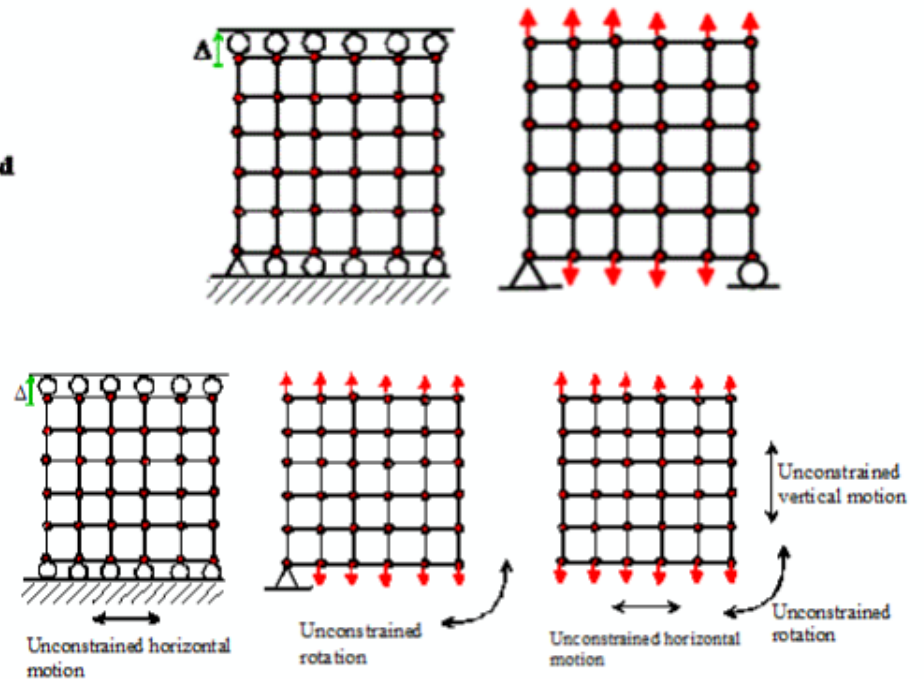
Boundary Conditions

We can apply

1. Prescribed displacements
2. Forces on nodes
3. Pressure on element faces
4. Body forces
5. For some elements, can apply rotations/moments



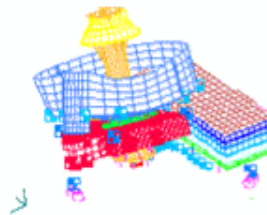
Properly constrained solids



Incorrectly constrained solids

Constraints Equations that relate DOF

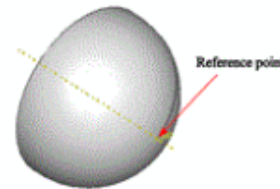
Shell elements connected to solid
 We would need to connect the shell to the solid with a constraint



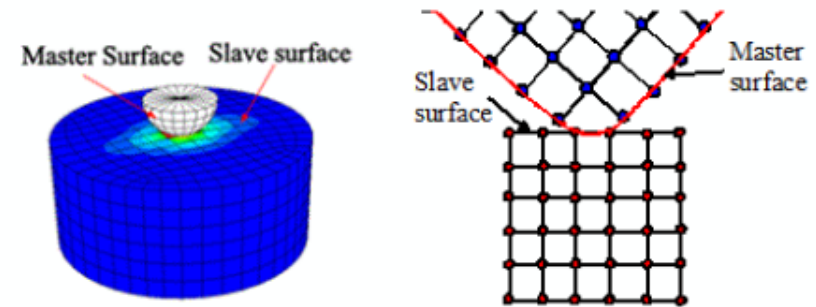
Contact

Select

1. Contact algorithm
2. Constitutive law for contact (eg friction)
3. "Soft" or "Hard" contact



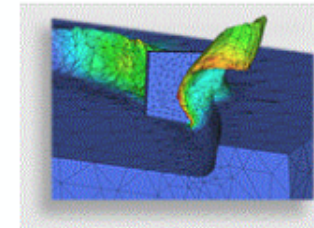
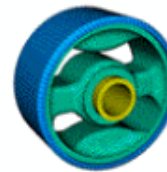
Master/slave pairs



Nodes on slave surface are prevented from penetrating inside master surface

Solution Procedures

Small strain –v- large strain (NLGEOM)



Static analysis

Solves $\mathbf{R}(\mathbf{u}) = \mathbf{F}^*$ $\mathbf{u} = \mathbf{u}^*$ using Newton-Raphson iteration

Explicit Dynamics: solves $\mathbf{M}\ddot{\mathbf{u}} + \mathbf{R}(\mathbf{u}) = \mathbf{F}^*$ $\mathbf{u} = \mathbf{u}^*$ using 2nd order forward Euler scheme

Implicit Dynamics: solves $\mathbf{M}\ddot{\mathbf{u}} + \mathbf{R}(\mathbf{u}) = \mathbf{F}^*$ $\mathbf{u} = \mathbf{u}^*$ using 2nd order backward Euler scheme

Special procedures: modal dynamics, buckling ('Linear Perturbation steps')

Topics for today's class

- Implicit dynamics
- Units in FEA
- Using dimensionless variables in simulations; scaling governing equations

- Brief look at homework 1

- FEA for static linear elasticity
 - Governing equations for elastostatics
 - The principle of minimum potential energy; estimating displacement fields by minimizing energy
 - Simple FEA program for linear elasticity – 2D plane strain with constant strain triangular elements

2.6.2 Implicit dynamics

- Solves $F = ma$ using backward-Euler :

$$(a) \quad \underline{v}(t+\Delta t) = \underline{v}(t) + \underline{a} \Delta t$$

$$(b) \quad \underline{u}(t+\Delta t) = \underline{u}(t) + \underline{v}(t+\Delta t) \Delta t + \underline{a} \Delta t^2 / 2$$

$$(c) \quad \underline{a} = [M]^{-1} (\underline{F}^* - R(\underline{u}(t+\Delta t)))$$

Solve (a-c) for $\underline{u}(t+\Delta t)$

Unconditionally stable - Δt can be chosen freely
 - Does not conserve energy for large Δt

- Δt may need to be small for accuracy

2.7 Units and scaling in FEA

FEA is solving $F = ma$ - no units a-priori
- any consistent set is fine

Be careful with length - if we sketch a part in mm or cm, that sets length unit

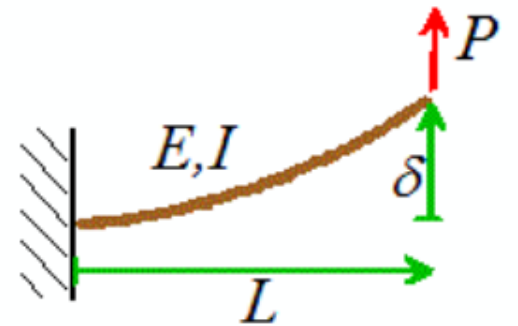
If force is in N, stress is in N/mm^2 or N/cm^2

2.7.1 Using scaling / dimensional analysis

Example: Cantilever beam

$$\delta = f(P, E, I, L) \leftarrow 4 \text{ variables}$$

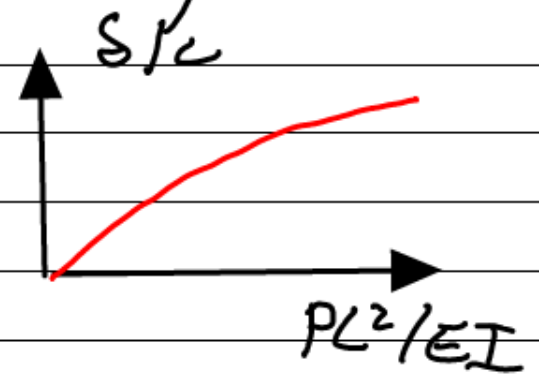
$$\frac{\delta}{L} = g\left(\frac{P}{EI^2}, \frac{I}{L^4}\right) \leftarrow 2 \text{ variables}$$



If we know solution only depends on product EI then can simplify further.

$$\delta/L = h \left(PL^2/EI \right)$$

If we solved a linear problem
 $\delta \propto P$



$$\Rightarrow \delta/L = \beta \frac{PL^2}{EI} \quad \beta \text{ is a constant}$$

-now just need 1 FEA simulation

This approach works but a better method is to scale governing equations

For beam problems we solve for deflection $w(x)$

• Eqs: $EI w^{IV} = 0$

• BCs: $\left. \begin{array}{l} w = 0 \\ w' = 0 \end{array} \right\} x=0$ $\left. \begin{array}{l} w'' = 0 \\ EI w''' = P \end{array} \right\} x=L$

Let $w = L \hat{w}(\xi)$ $x = \xi L$ $0 < \xi < 1$ $\frac{\partial}{\partial x} = \frac{1}{L} \frac{\partial}{\partial \xi}$

Governing eqs are now

$\frac{EI}{L^3} \frac{\partial^4 \hat{w}}{\partial \xi^4} = 0$

$\left. \begin{array}{l} \hat{w} = 0 \\ \frac{\partial \hat{w}}{\partial \xi} = 0 \end{array} \right\} \xi = 0$

$\left. \begin{array}{l} \frac{\partial^2 \hat{w}}{\partial \xi^2} = 0 \\ \frac{EI}{L^2} \frac{\partial^3 \hat{w}}{\partial \xi^3} = P \end{array} \right\} \xi = 1$

Combine

Hence solution depends on $\frac{PL^2}{EI}$

We can simplify further by defining

$$\omega = \frac{EI}{PL^2} \hat{w}$$

Governing eqs $\partial^4 \omega / \partial \xi^4 = 0$

$$\left. \begin{array}{l} \text{BCs} \quad \left. \begin{array}{l} \partial \omega / \partial \xi = 0 \\ \omega = 0 \end{array} \right\} \xi = 0 \quad \left. \begin{array}{l} \partial^2 \omega / \partial \xi^2 = 0 \\ \frac{\partial^3 \omega}{\partial \xi^3} = 1 \end{array} \right\} \xi = 1 \end{array} \right\}$$

ω applies to all beams!

To find $\delta = \frac{PL^3}{EI} \omega(\xi=1)$

Preview – Basic FEA for static linear elasticity

Background: Governing equations for linear elasticity The Principle of Minimum Potential Energy

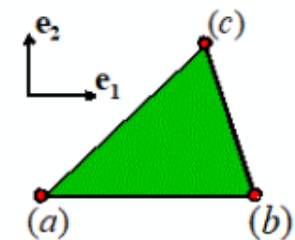
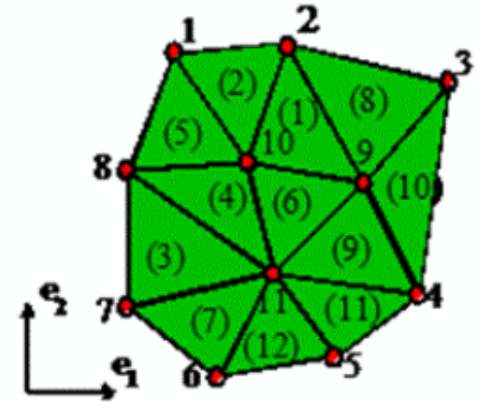
- Plane strain linear elasticity problem as an energy minimization

$$\Phi = \int_A \phi dA - \int_{S_2} \mathbf{t}^* \cdot \mathbf{u} ds$$

- Interpolating the displacement field using constant strain triangles
- Computing the potential energy
 - Strains, stresses and strain energy density inside an element.
 - Element stiffness matrix
 - Potential energy of a loaded element face – element force vector
 - Total potential energy – global stiffness and global force
 - Constrained boundary conditions
- Minimizing the total potential energy

$$\Phi = \frac{1}{2} (\mathbf{u}^{Global})^T [\mathbf{K}] \mathbf{u}^{Global} - (\mathbf{u}^{Global})^T \cdot \mathbf{f}^{Global} \Rightarrow [\mathbf{K}] \mathbf{u}^{Global} = \mathbf{f}^{Global}$$

- Implementation
 - Data structures for mesh and BC definition
 - Assembling the element and global stiffness matrices
 - Prescribing boundary conditions
 - Solution and post-processing



3 FEA for plane linear elastostatics

Background: Governing Equations

Find : $[u_i, \epsilon_{ij}, \sigma_{ij}]$

Satisfying

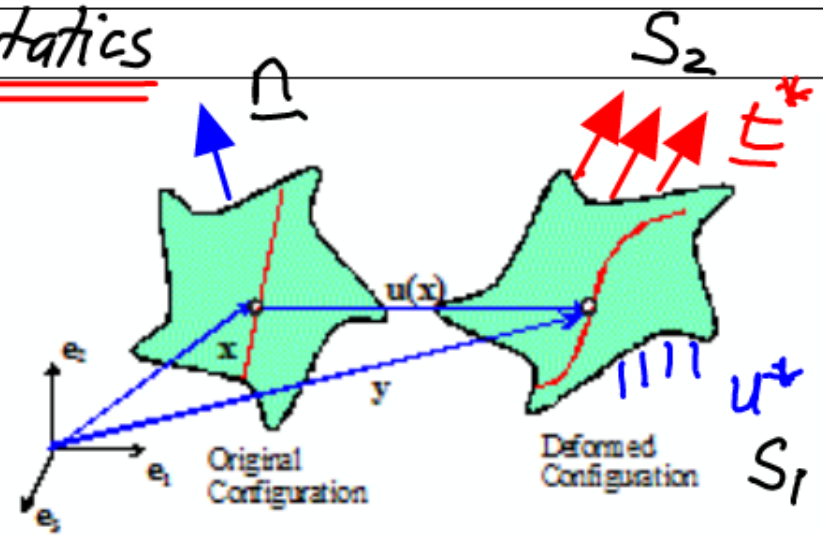
$$\textcircled{1} \quad \epsilon_{ij} = \frac{1}{2} \left(\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right)$$

$$\textcircled{2} \quad \sigma_{ij} = C_{ijkl} \epsilon_{kl}$$

$$\textcircled{3} \quad \frac{\partial \sigma_{ij}}{\partial x_i} = 0 \quad \sigma_{ij} = \sigma_{ji}$$

$$\textcircled{4} \quad u_i = u_i^* \text{ on } S_1$$

$$C_{ijkl} = \frac{E}{2(1+\nu)} (\delta_{il}\delta_{jk} + \delta_{ik}\delta_{jl}) + \frac{E\nu}{(1+\nu)(1-2\nu)} \delta_{ij}\delta_{kl}$$



E - Young's modulus
 ν - Poisson's ratio

$$\sigma_{ij} n_i = t_j^* \text{ on } S_2$$

Principle of minimum potential energy

Let v_i be a differentiable vector field on \mathcal{R}

$$\text{Let } \hat{\epsilon}_{ij} = \frac{1}{2} \left(\frac{\partial v_i}{\partial x_j} + \frac{\partial v_j}{\partial x_i} \right)$$

Strain energy density $\phi = \frac{1}{2} C_{ijkl} \hat{\epsilon}_{ij} \hat{\epsilon}_{kl}$

$$\text{Define total PE } \Phi = \int_V \phi dV - \int_{S_2} t_i^* v_i dA$$

$$\text{Then } \Phi(\underline{v}) \geq \Phi(\underline{u})$$

$$\text{and } \Phi(\underline{v}) = \Phi(\underline{u}) \Leftrightarrow \underline{v} = \underline{u}$$

Application: Approximate \underline{u} in some way.
Get best approximation by minimizing Φ

3.1) Plane linear elastostatic FE with constant strain triangular elements

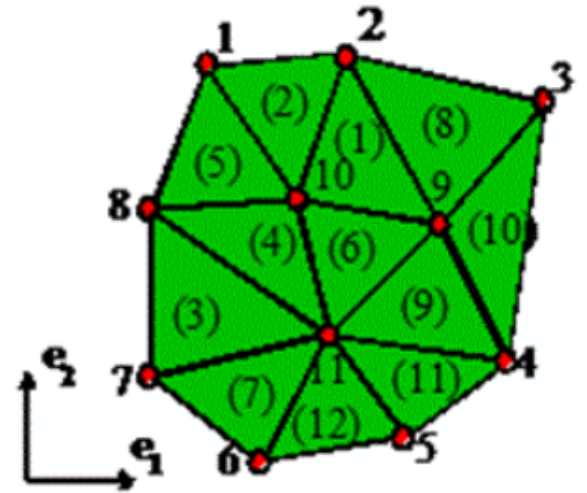
(a) Divide solid into triangles

(b) Let u_i^a denote unknown displacement
@ nodes $a = 1 \dots N$

(c) Interpolate \underline{u} inside each triangle
eg for a point inside triangle
with vertices a, b, c

$$\underline{u} = N^a(\underline{x}) \underline{u}^a + N^b(\underline{x}) \underline{u}^b + N^c(\underline{x}) \underline{u}^c$$

(d) Find Φ ; minimize wrt \underline{u}^a

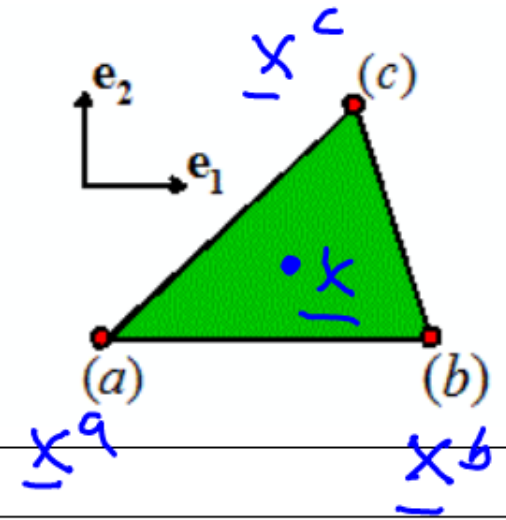


• Interpolation functions for constant strain triangle

$$N^a(x_1, x_2) = \frac{(x_2 - x_2^{(b)})(x_1^{(c)} - x_1^{(b)}) - (x_1 - x_1^{(b)})(x_2^{(c)} - x_2^{(b)})}{(x_2^{(a)} - x_2^{(b)})(x_1^{(c)} - x_1^{(b)}) - (x_1^{(a)} - x_1^{(b)})(x_2^{(c)} - x_2^{(b)})}$$

$$N^b(x_1, x_2) = \frac{(x_2 - x_2^{(c)})(x_1^{(a)} - x_1^{(c)}) - (x_1 - x_1^{(c)})(x_2^{(a)} - x_2^{(c)})}{(x_2^{(b)} - x_2^{(c)})(x_1^{(a)} - x_1^{(c)}) - (x_1^{(b)} - x_1^{(c)})(x_2^{(a)} - x_2^{(c)})}$$

$$N^c(x_1, x_2) = \frac{(x_2 - x_2^{(a)})(x_1^{(b)} - x_1^{(a)}) - (x_1 - x_1^{(a)})(x_2^{(b)} - x_2^{(a)})}{(x_2^{(c)} - x_2^{(a)})(x_1^{(b)} - x_1^{(a)}) - (x_1^{(c)} - x_1^{(a)})(x_2^{(b)} - x_2^{(a)})}$$



Notice etc $N^a = 1$ $N^b = N^c = 0$ @ $\underline{x} = \underline{x}^a$

Interpolations are linear functions of position

