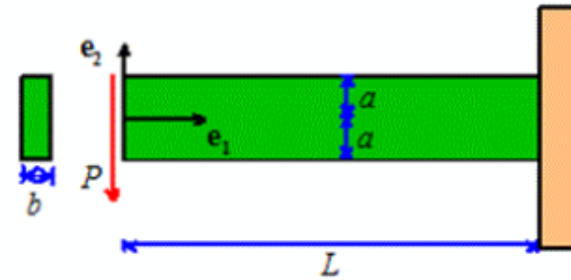


# Review: Shear Locking

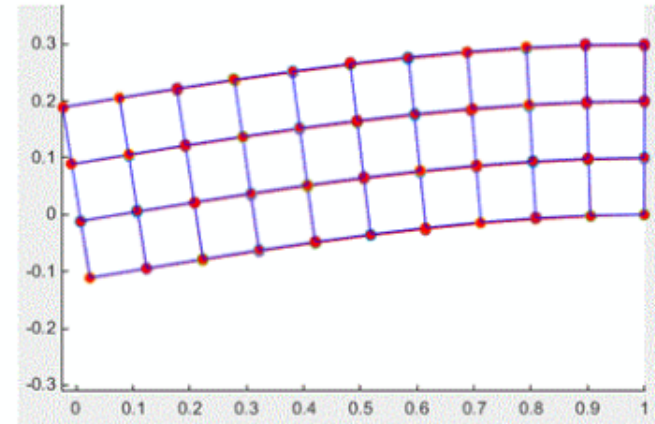
Analytical solution to 2D elasticity problem  
 (See Chapter 5 of [solidmechanics.org](http://solidmechanics.org) for details)

$$u_1 = \frac{3P}{4Ea^3b} x_1^2 x_2 - \frac{P}{4Ea^3b} (2+\nu)x_2^3 + \frac{3P}{2Ea^3b} (1+\nu)a^2 x_2 - \frac{3PL^2 x_2}{4Ea^3b}$$

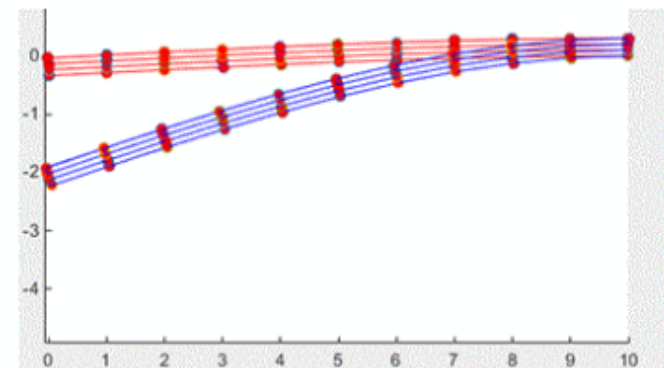
$$u_2 = -\nu \frac{3P}{4Ea^3b} x_1 x_2^2 - \frac{P}{4Ea^3b} x_1^3 + \frac{3PL^2 x_1}{4Ea^3b} - \frac{PL^3}{2Ea^3b}$$



Solution for short beam is OK (elements are square)



Linear elements underestimate solution by large factor  
 Caused by "Shear Locking"



## Topics for today's class

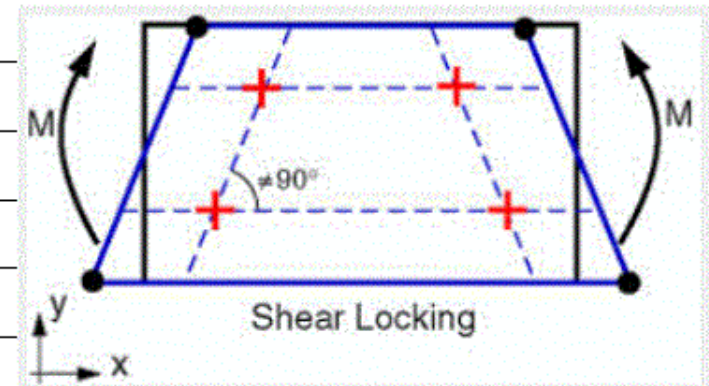
- Problems with standard elements
  - 'Shear Locking' in beam/plate/shell problems
  - 'Volumetric locking' in near-incompressible materials

• Standard linear elements cannot interpolate bending fields

• Bending linear elements induces a large spurious shear strain at integration points

- Spuriously stiff beam

- Fix with "Incompatible mode" elements



• ABAQUS :

Element Type

Element Library:  Standard  Explicit

Geometric Order:  Linear  Quadratic

Family: Acoustic, Beam Section, Cohesive, Cohesive Pore Pressure

Quad  Tri

Reduced integration  Incompatible modes

Element Controls

Hourglass stiffness:  Use default  Specify [ ]

Viscosity:  Use default  Specify [ ]

Second-order accuracy:  Yes  No

Distortion control:  Use default  Yes  No

Length ratio: 0.1

Hourglass control:  Use default  Enhanced  Relax stiffness  Stiffness  Viscous  Combined

CPS4R: A 4-node bilinear plane stress quadrilateral reduced integration hourglass control.

Element Type

Element Library:  Standard  Explicit

Geometric Order:  Linear  Quadratic

Family: Acoustic, Beam Section, Cohesive, Cohesive Pore Pressure

Quad  Tri

Reduced integration

Element Controls

Viscosity:  Use default  Specify [ ]

Element deletion:  Use default  Yes  No

Max Degradation:  Use default  Specify [ ]

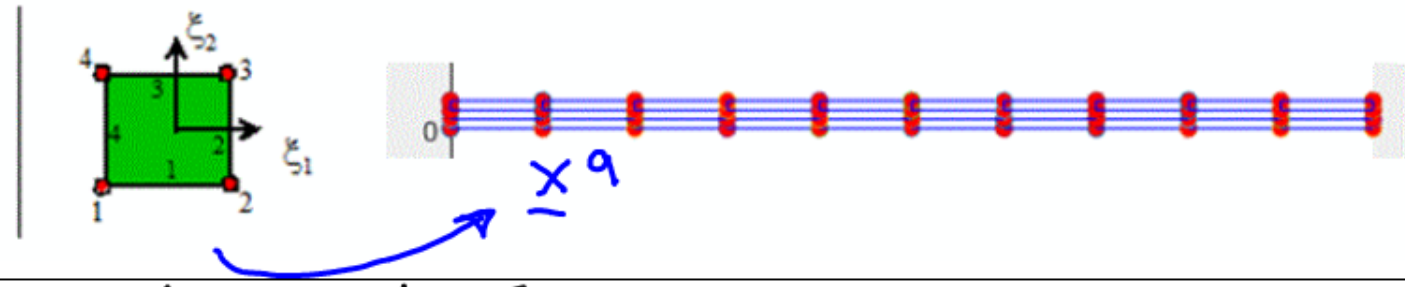
Fix is to introduce additional DOF to interpolate strains inside element

Describe displacements with standard isoparametric mapping

$$\underline{u} = N^a(\underline{\xi}) \underline{u}^a$$

$$\underline{x} = N^a(\underline{\xi}) \underline{x}^a$$

- $N^1 = 0.25(1 - \xi_1)(1 - \xi_2)$
- $N^2 = 0.25(1 + \xi_1)(1 - \xi_2)$
- $N^3 = 0.25(1 + \xi_1)(1 + \xi_2)$
- $N^4 = 0.25(1 - \xi_1)(1 + \xi_2)$



Modify displacement gradient

$$\frac{\partial u_i}{\partial x_j} = \left\{ \frac{\partial N^a}{\partial \xi_m} u_i^a + \sum_{k=1}^p \frac{\eta(0)}{\eta(\underline{\xi})} \alpha_i \delta_{km} \xi_k \right\} \frac{\partial \xi_m}{\partial x_j}$$

new element DOFs

$p = \# \text{ coords}$

$$\eta = \det \left( \frac{dx}{d\xi} \right)$$



Interpolate virtual displacement  $\delta u_i$  and gradient the same way:

$$\frac{\partial \delta u_i}{\partial x_j} = \left\{ \frac{\partial N^a}{\partial \xi_m} \delta u_i^a + \sum_{k=1}^P \frac{\eta^{(k)}}{\eta(\xi)} \delta \alpha_i^{(k)} \delta_{km} \xi_k \right\} \frac{\partial \xi_m}{\partial x_j}$$

PVW:

$$\int_{\mathcal{R}} C_{ijkl} \frac{\partial u_k}{\partial x_l} \frac{\partial \delta u_i}{\partial x_j} dV + \int_{\mathcal{R}} \sigma_{ij}^0 \frac{\partial \delta u_i}{\partial x_j} dV - \int_{S_r} t_i^* \delta u_i dA = 0$$

New discrete system has form

$$\begin{aligned} & \left[ K_{aibk}^{uu} U_k^b + K_{aike}^{u\alpha} \alpha_e^{(k)} - R_{ai}^u - f_{ia} \right] \delta u_i^a = 0 \\ & \left[ K_{ijbk}^{\alpha u} U_k^b + K_{ijke}^{\alpha\alpha} \alpha_e^k - R_{ij}^\alpha \right] \delta \alpha_j^i = 0 \end{aligned}$$

Must hold  $\forall$  admiss  $\delta u_i^a$  and  $\delta \alpha_j^i$   
 $\Rightarrow$  New system of FEA eqs

## Implementing as code

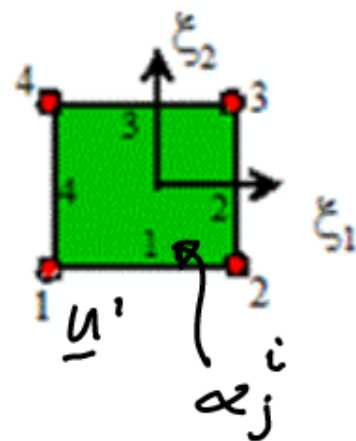
Focus on element level operations  
(Illustrate in 2D)

Define new B matrix

$$\begin{bmatrix} \varepsilon_{11} \\ \varepsilon_{22} \\ \varepsilon_{12} \end{bmatrix} = [B] \begin{bmatrix} \underline{u} \\ \underline{\alpha} \end{bmatrix}$$

$$\underline{u} = [u_1^1, u_2^1, \dots, u_1^4, u_2^4]$$

$$\underline{\alpha} = [\alpha_1^1, \alpha_2^1, \alpha_1^2, \alpha_2^2]$$



$$[B] = \begin{bmatrix} \frac{\partial N^1}{\partial x_1} & 0 & \frac{\partial N^2}{\partial x_1} & 0 & \frac{\partial N^3}{\partial x_1} & 0 & \frac{\eta(0)}{\eta(\xi)} \xi_1 \frac{\partial \xi_1}{\partial x_1} & 0 & \frac{\eta(0)}{\eta(\xi)} \xi_2 \frac{\partial \xi_2}{\partial x_1} & 0 \\ 0 & \frac{\partial N^1}{\partial x_2} & 0 & \frac{\partial N^2}{\partial x_2} & 0 & \frac{\partial N^3}{\partial x_2} & 0 & \frac{\eta(0)}{\eta(\xi)} \xi_1 \frac{\partial \xi_1}{\partial x_2} & 0 & \frac{\eta(0)}{\eta(\xi)} \xi_2 \frac{\partial \xi_2}{\partial x_2} \\ \frac{\partial N^1}{\partial x_2} & \frac{\partial N^1}{\partial x_1} & \frac{\partial N^2}{\partial x_2} & \frac{\partial N^2}{\partial x_1} & \frac{\partial N^3}{\partial x_2} & \frac{\partial N^3}{\partial x_1} & \frac{\eta(0)}{\eta(\xi)} \xi_1 \frac{\partial \xi_1}{\partial x_2} & \frac{\eta(0)}{\eta(\xi)} \xi_1 \frac{\partial \xi_1}{\partial x_1} & \frac{\eta(0)}{\eta(\xi)} \xi_2 \frac{\partial \xi_2}{\partial x_2} & \frac{\eta(0)}{\eta(\xi)} \xi_2 \frac{\partial \xi_2}{\partial x_1} \end{bmatrix}$$

Now define

$$\underbrace{[\hat{K}]}_{12 \times 12} = \int_{\Omega_e} [B]^T [D] [B] dV$$

$$R = - \int_{\Omega_e} [B]^T \underline{\sigma}^0 dV$$

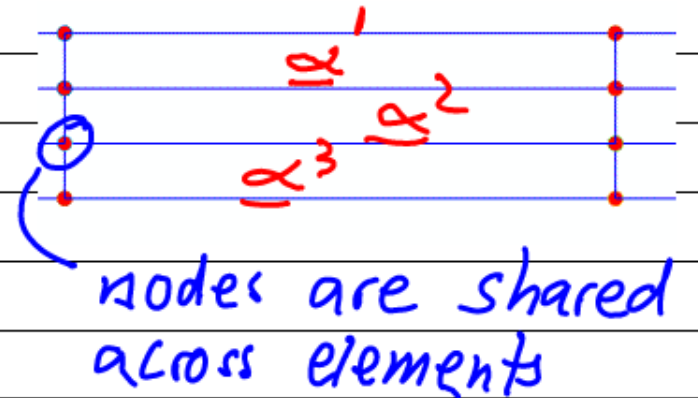
$\hat{K}$  has the following structure;

$$\begin{bmatrix} \underbrace{K^{uu}}_{8 \times 8} & \underbrace{K^{u\alpha}}_{8 \times 4} \\ \underbrace{K^{\alpha u}}_{4 \times 8} & \underbrace{K^{\alpha\alpha}}_{4 \times 4} \end{bmatrix} \begin{bmatrix} u \\ \alpha \end{bmatrix} = \begin{bmatrix} 17 \\ 5 \\ 17 \\ 2 \end{bmatrix}$$

Notice that  $\alpha$  variables are local to each element

This means we can eliminate  $\alpha$  inside element

- no need to add to global system



$$\text{Elimination : } \underline{\alpha} = K^{\alpha\alpha-1} \left\{ \underline{\Gamma}^{\alpha} - K^{\alpha u} \underline{u} \right\} \quad (\text{last 4 rows})$$

Subst back

$$\underbrace{\left\{ K^{uu} - K^{u\alpha} K^{\alpha\alpha-1} K^{\alpha u} \right\}}_{[K^{el}] \quad 8 \times 8} \underline{u} = \underbrace{\left\{ \underline{\Gamma}^u - K^{\alpha\alpha-1} \underline{\Gamma}^{\alpha} \right\}}_{8 \times 1 \text{ vector} \quad \underline{\Gamma}^{el}}$$



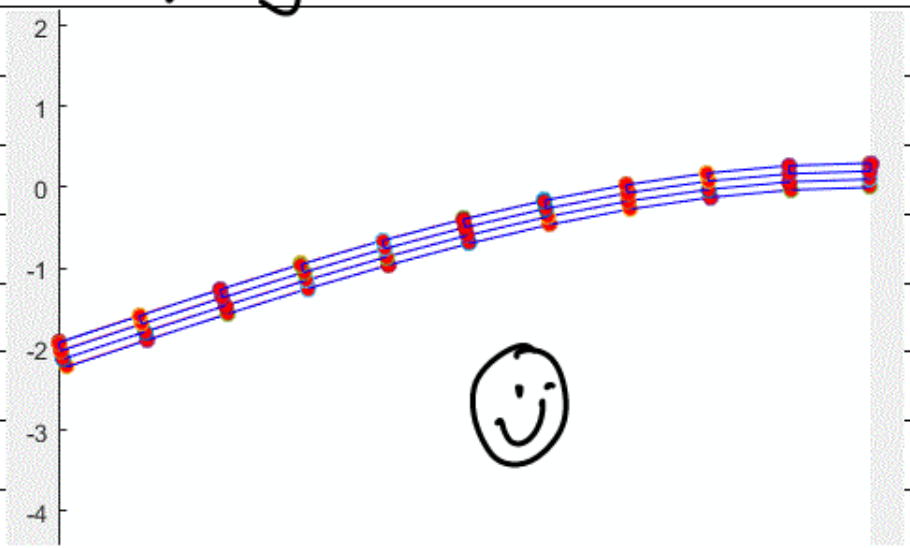
Summary:

- ① Assemble  $[\hat{K}]$  - use standard code, loop over integration points, etc
- ② Extract  $k^{uu}$   $k^{ua}$   $k^{au}$   $k^{aa}$
- ③ Assemble  $k^{el}$

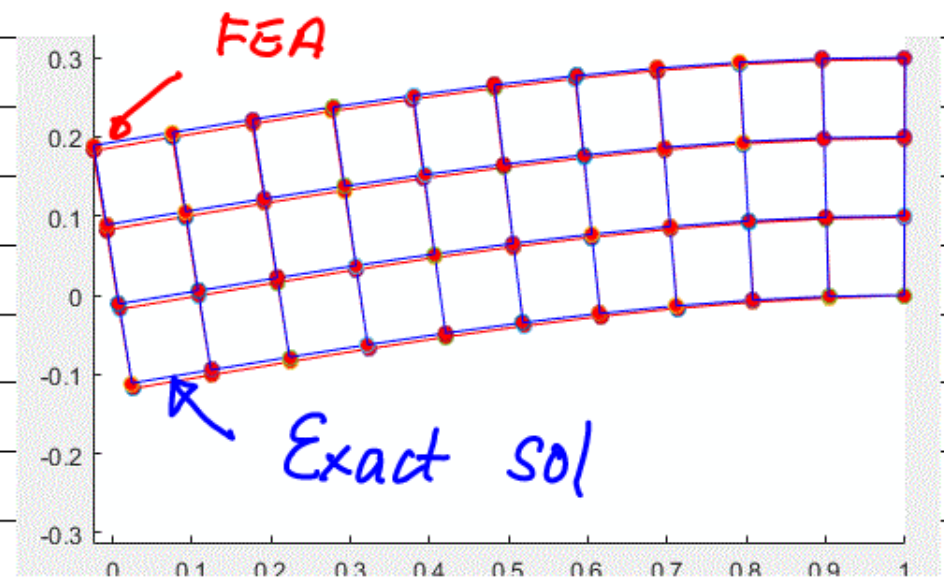
Remainder of code is unaffected

Test beam problem again

Long beam



Short beam.



This can be generalized to construct a family of "assumed strain" elements

Construct with care: Added strain DOF must be independent

Can perform poorly for finite deformation

## 6.2) Volumetric Locking in near-incompressible material

Solve pressurized cylinder problem

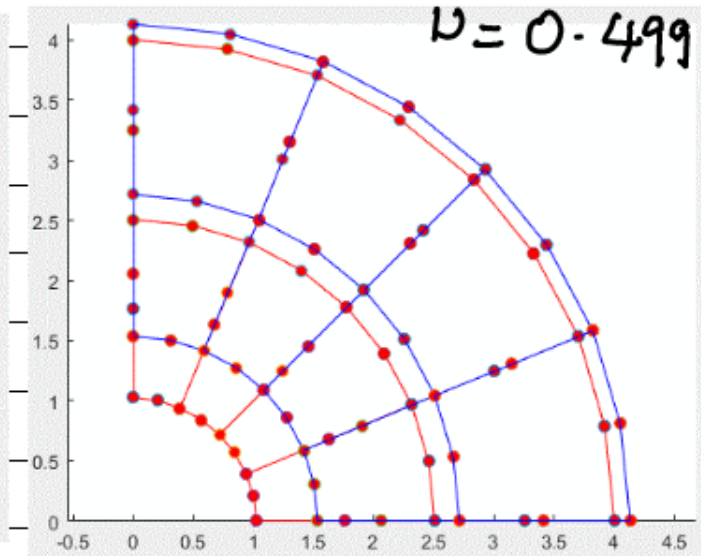
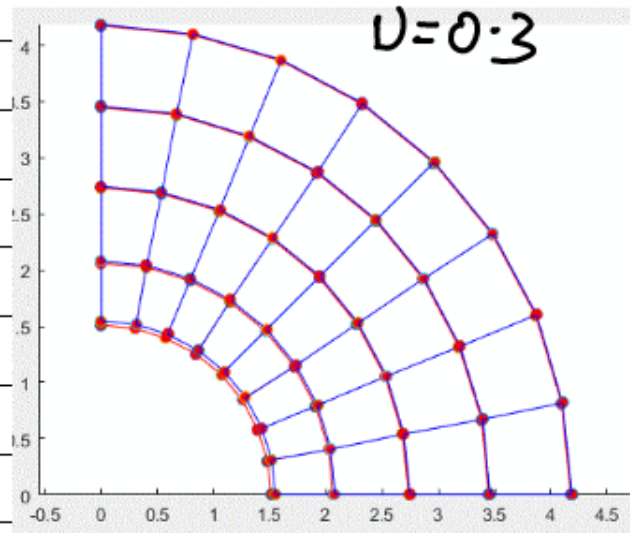
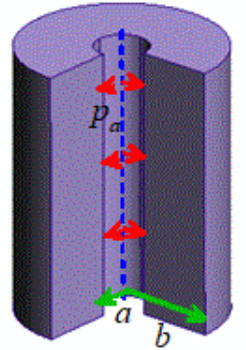
Use fully integrated 2D plane strain elements

Good accuracy for  $\nu = 0.3$  with all element types

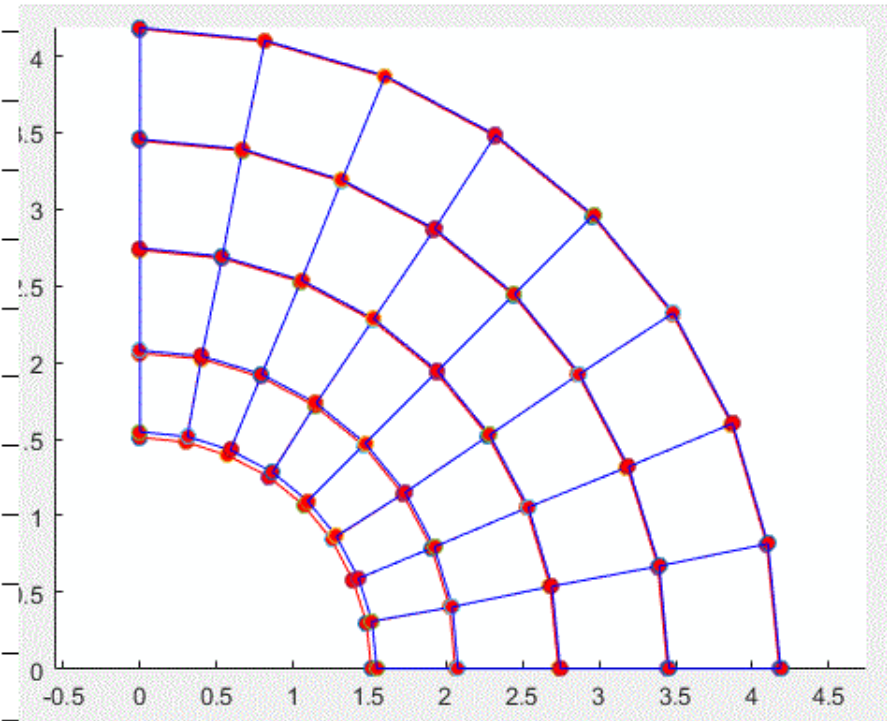
For  $\nu$  close to  $\frac{1}{2}$  FEA displacements too small in all element types

Analytical solution to 2D elasticity problem  
(See Chapter 4 of [solidmechanics.org](http://solidmechanics.org) for details)

$$\mathbf{u} = \frac{(1+\nu)a^2b^2}{E(b^2-a^2)} \left\{ \frac{p}{r} + (1-2\nu)\frac{p}{b^2}r \right\} \mathbf{e}_r$$



This is produced by "volumetric locking"



We need to satisfy  $\epsilon_{kk} \approx 0$   
at every integration point

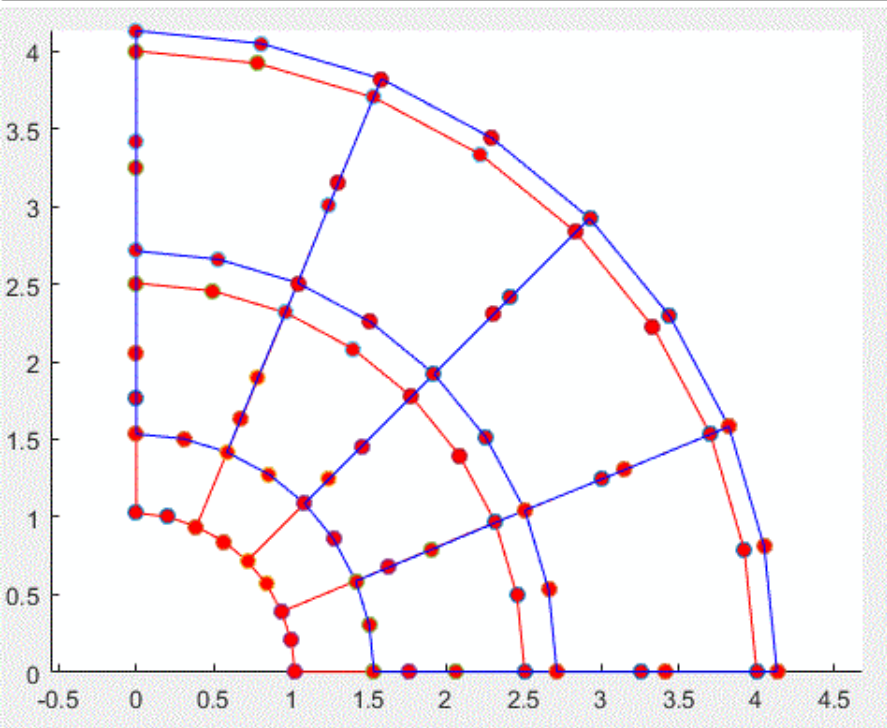
4 int pts per element for  
linear quads  $\Rightarrow$  128 constraints

80 DOF ( $\# \text{ nodes} \times \# \text{ DOF}$   
-  $\# \text{ constrained DOF}$ )

Far more constraints than DOF

Only solution with  $\epsilon_{kk} \approx 0$  is  
 $u \approx 0$

# Quadratic



64 DOF

9 int pts per element

$8 \times 9$  constraints

- same problem !

Fixes:

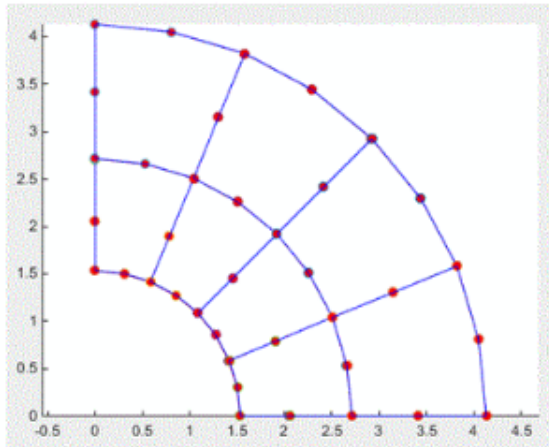
- (1) Reduced Integration.
- (2) "Selective" reduced integration.
- (3) "B-bar" method.
- (4) Hybrid, or mixed elements.



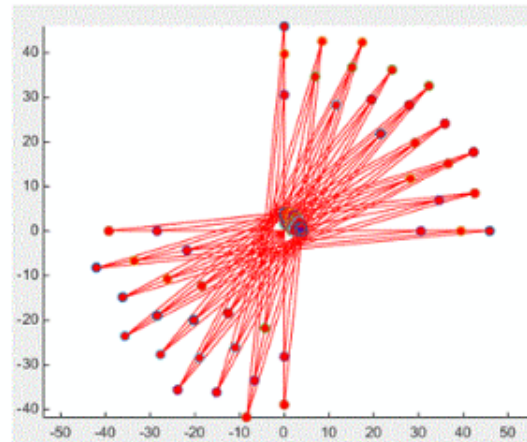
# Fix #1 "Reduced Integration" elements

- try reducing # int pts (4 for quadratic, 1 for linear)

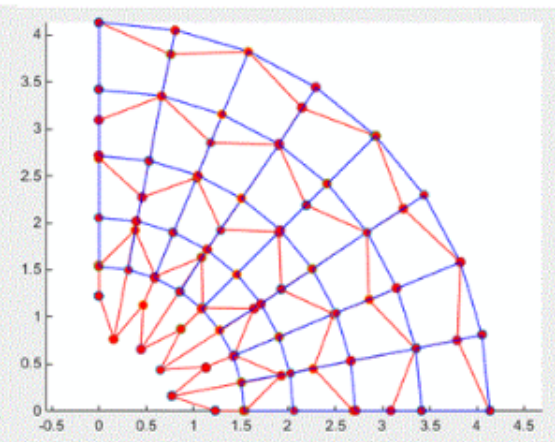
Number of integration points for reduced integration schemes	
Linear triangle (3 nodes): 1 point	Linear tetrahedron (4 nodes): 1 point
Quadratic triangle (6 nodes): 3 points	Quadratic tetrahedron (10 nodes): 4 points
Linear quadrilateral (4 nodes): 1 point	Linear brick (8 nodes): 1 point
Quadratic quadrilateral (8 nodes): 4 points	Quadratic brick (20 nodes): 8 points



8 noded quads – perfect!



4 noded quads – Hourglassing



Scaled to show hourglass mode