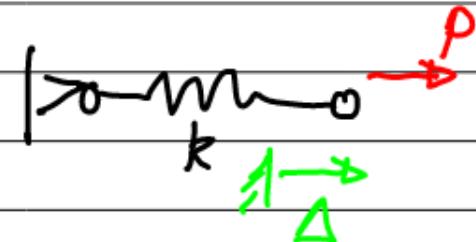


page 1 9 Analyzing elastic solids using energy

Background : we can calculate deflection of a spring by minimizing its PE



$$PE: T = \frac{1}{2} k \Delta^2 - P \Delta$$

$$\text{Minimize wrt } \Delta : \frac{\partial T}{\partial \Delta} = k \Delta - P = 0 \Rightarrow \Delta = \frac{P}{k}$$

Same idea works for continuum

- Applications : use energy to get approximate solutions
- : energy can be used to derive FEA equations

page 2 9.1 Definition of PE for elastic solids

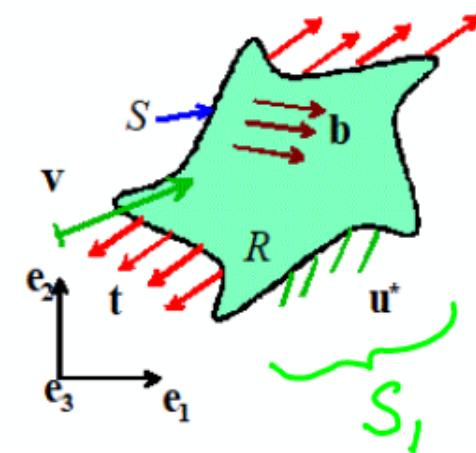
Assumptions : (1) Elastic solid ; small deformation

(2) No temp changes

(3) Static equilibrium

(4) Boundary conditions  $\underline{u} = \underline{u}^*$  on  $S_1$

$$\underline{\sigma} = \underline{\epsilon} \text{ on } S_2$$



Exact solution  $[\underline{u}, \underline{\epsilon}, \underline{\sigma}]$

Approximation for displacement

Let  $\hat{\underline{u}}(\underline{x})$  be a guess for displacement satisfying

$\hat{\underline{u}} = \underline{u}^*$  on  $S_1$  "kinematically admissible displacement field"

$$\text{Strain } \hat{\underline{\epsilon}} = [\nabla \hat{\underline{u}} + (\nabla \hat{\underline{u}})^T]/2$$

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## Definition of PE

Strain energy density  $U = \frac{1}{2} \hat{\underline{\sigma}} \cdot \hat{\underline{\varepsilon}} = \frac{1}{2} \hat{\underline{\varepsilon}} \cdot ([C] \hat{\underline{\varepsilon}})$

$$\hat{\underline{\varepsilon}} = [\hat{\varepsilon}_{11}, \hat{\varepsilon}_{22}, \hat{\varepsilon}_{33}, 2\hat{\varepsilon}_{12}, 2\hat{\varepsilon}_{13}, 2\hat{\varepsilon}_{23}]$$

$$\hat{\underline{\sigma}} = [\sigma_{11}, \sigma_{22}, \sigma_{33}, \sigma_{12}, \sigma_{13}, \sigma_{23}, \text{etc.}]$$

elastic  
constants

Define PE

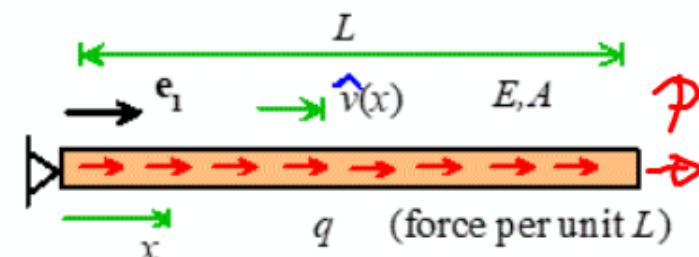
$$\Pi(\hat{\underline{v}}) = \int_V \bar{U} d\bar{V} - \int_V \rho \underline{b} \cdot \hat{\underline{v}} dV - \int_{S_2} \underline{t} \cdot \hat{\underline{v}} dA$$

Example: 1D bar

$$\hat{\underline{\varepsilon}} = \frac{d\hat{\underline{v}}}{dx}$$

$$\sigma = E \hat{\underline{\varepsilon}}$$

$$\bar{U} = \frac{1}{2} \sigma \hat{\underline{\varepsilon}} \\ = \frac{1}{2} E \left( \frac{d\hat{\underline{v}}}{dx} \right)^2$$



$$\sigma_{11} \neq 0 \quad \text{all others } 0$$

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page 4

$$\text{Hence } \Pi = \int_0^L \frac{1}{2} A E \left( \frac{d\hat{u}}{dx} \right)^2 dx - \int_0^L q \hat{u} dx - P \hat{u}(L)$$

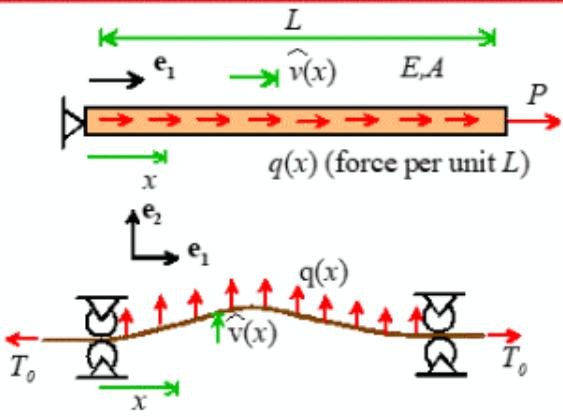
Can calculate  $\Pi$  for many other geometries

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## Useful formulas for potential energy

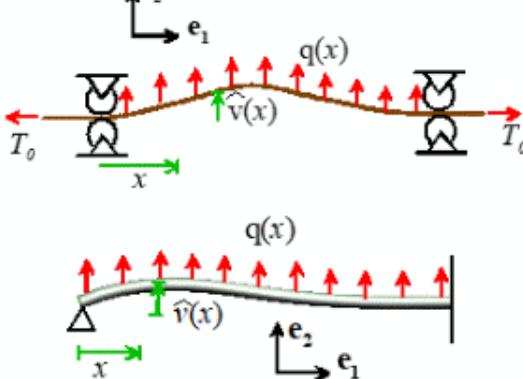
1-D axially loaded bar

$$\Pi = \int_0^L \frac{1}{2} EA \left( \frac{d\hat{v}}{dx} \right)^2 dx - \int_0^L q(x) \hat{v}(x) dx - Pv(L)$$



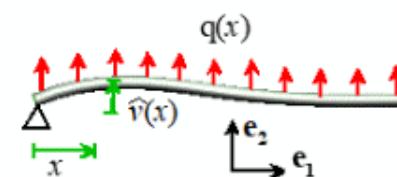
1-D Tensioned cable

$$\Pi = T_0 \int_0^L \frac{1}{2} \left( \frac{d\hat{v}}{dx} \right)^2 dx - \int_0^L q(x) \hat{v}(x) dx$$



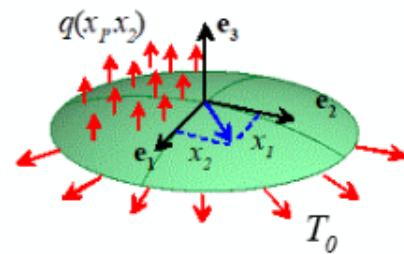
1-D Euler-Bernoulli beam

$$\Pi = \int_0^L \frac{1}{2} EI \left( \frac{d^2\hat{v}}{dx^2} \right)^2 dx - \int_0^L q(x) \hat{v}(x) dx$$



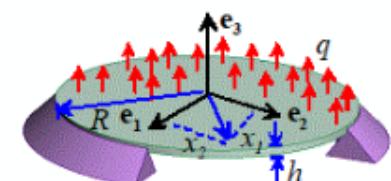
2-D biaxially stretched membrane

$$\Pi = \int_A \frac{1}{2} T_0 \left[ \left( \frac{\partial \hat{v}}{\partial x_1} \right)^2 + \left( \frac{\partial \hat{v}}{\partial x_2} \right)^2 \right] dA - \int_A q(x_1, x_2) \hat{v}(x_1, x_2) dA$$



2-D Kirchhoff plate

$$\Pi = \frac{Eh^3}{12(1-\nu)} \int_A \frac{1}{2} \left[ \left( \frac{\partial^2 \hat{v}}{\partial x_1^2} + \frac{\partial^2 \hat{v}}{\partial x_2^2} \right)^2 - 2(1-\nu) \left( \frac{\partial^2 \hat{v}}{\partial x_1^2} \frac{\partial^2 \hat{v}}{\partial x_2^2} - \left( \frac{\partial^2 \hat{v}}{\partial x_1 \partial x_2} \right)^2 \right) \right] dA - \int_A q(x_1, x_2) \hat{v}(x_1, x_2) dA$$



### 9.3 Principle of minimum potential energy

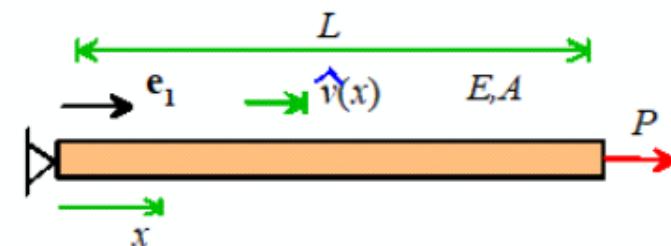
Among all possible guesses  $\hat{u}$  the exact solution [satisfies stress equilibrium] has minimum PE

$$\Pi(\hat{u}) > \Pi(u)$$

Example: 1D bar with axial load

Exact solution  $\sigma = \frac{P}{A}$   $\epsilon = \sigma/E$

$$\Rightarrow \frac{du}{dx} = \epsilon \Rightarrow u = \frac{P}{AE} x$$



$$\Pi(u) = \int_0^L \frac{1}{2} EA \left( \frac{du}{dx} \right)^2 dx - P \frac{PL}{AE} = -\frac{P^2 L}{2AE}$$

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Guess  $\hat{u} = \beta x$  ( $\beta$  some constant)

$$\Pi(\hat{u}) = \int_0^L \frac{1}{2} EA \left( \frac{d\hat{u}}{dx} \right)^2 dx - P\beta L = \frac{1}{2} EA \beta^2 - PL\beta$$

$$\text{Hence } \Pi(\hat{u}) - \Pi(u) = \frac{1}{2} LEA \beta^2 - PL\beta + \frac{P^2 L}{2AE}$$

$$= \frac{1}{2} EAL \left( \frac{\beta - P}{AE} \right)^2$$

$$\underbrace{\qquad}_{>0}$$

$$\text{Hence } \Pi(\hat{u}) > \Pi(u)$$

same idea used in general proof.

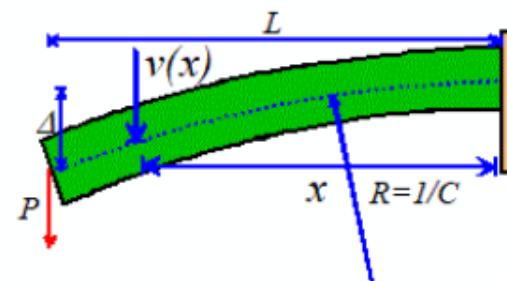
page 7

page 8 9.4 Applications of energy I : Estimating stiffness

Consider an example

**Example:** The beam has modulus  $E$  and moment of inertia  $I$ , and has stiffness  $k$  such that  $P = k\Delta$  ( $k_{exact} = 3EI / L^3$ )

Using  $\hat{v} = Cx^2 / 2$  as a guess for the displacement, estimate  $k$



Procedure:

(1) Find  $\Pi(u)$  in terms of  $k, P$   $\Pi = \frac{1}{2}k\Delta^2 - PA$

$$\Delta = P/k \Rightarrow \Pi(u) = -P^2/2k$$

(2) Find best (lowest) possible guess for  $\Pi(\hat{v})$

Using given guess  $\Pi(\hat{v}) = \int_0^L \frac{1}{2} EI \left( \frac{dv}{dx} \right)^2 dx - P\hat{v}(L)$

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$$\Rightarrow \Pi(\hat{v}) = \int_0^L \frac{EI}{2} C^2 dx - P \frac{C h^2}{2} = \frac{EILC^2}{2} - \frac{PCh^2}{2}$$

Minimize wrt  $C$  for best guess

$$\Rightarrow \frac{\partial \Pi}{\partial C} = 0 = EILC - \frac{PL^2}{2} = 0 \Rightarrow C = \frac{PL}{2EI}$$

$$\Rightarrow \Pi(\hat{v}) = -\frac{PL^3}{8EI}$$

$$\Pi(\hat{v}) > \Pi(u) \Rightarrow -\frac{PL^3}{8EI} > -\frac{P^2}{2k}$$

$$\Rightarrow k \leq \frac{4EI}{L^3}$$

"close enough  
for government  
work"

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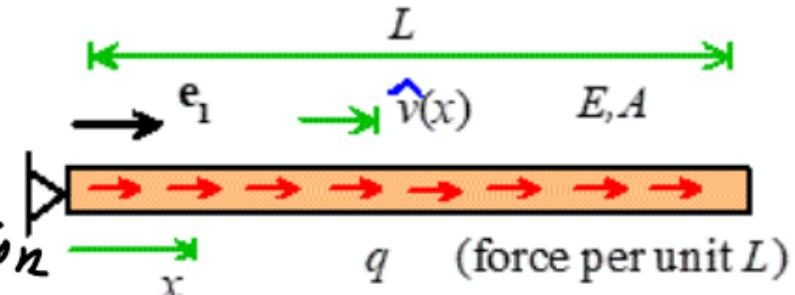
## 9.5 Applications, 2 : "Rayleigh - Ritz" approximation

General method for getting approximate solutions

Consider example (1D bar with body force)

Procedure: (1) Find a general approximation for  $\hat{v}$

$$\hat{v} = \sum_{i=1}^N a_i f_i(x)$$



where  $f_i(x)$  are "basis functions" eg  $f_i = x^{i-1}$   
 $f_i = \sin(\pi(i-1)x/L)$

$a_i$ : set of unknown coefficients TBD

page 11 To find  $a_i$  : (1) Eliminate some subset of  $a_i$  using boundary condition for  $\hat{v}$   
- get new  $\hat{v}$  with fewer unknowns

(2) Find  $\bar{\Pi}(a_i)$  ; minimize

$$\frac{\partial \bar{\Pi}}{\partial a_i} = 0 \leftarrow \begin{array}{l} \text{several eqs} \\ \text{- solve for } a_i \end{array}$$

For our example try  $\hat{v} = a_1 + a_2 x$

$$(1) \hat{v}(x=0) = 0 @ x=0 \Rightarrow a_1 = 0$$

$$\Rightarrow \hat{v} = a_2 x$$

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$$(2) \quad \Pi = \int_0^L \frac{EA}{2} \left( \frac{d\hat{v}}{dx} \right)^2 dx - \int_0^L q \hat{v} dx$$

$$\frac{EA}{2} a_2^2 L - \frac{1}{2} a_2 L^2 q$$

$$\text{Minimize} \Rightarrow \frac{\partial \Pi}{\partial a_2} = 0 \quad EA a_2 L - \frac{1}{2} L^2 q = 0$$

$$\Rightarrow a_2 = \frac{qL}{2EA}$$

$$\hat{v} = \frac{qL}{2EA} x$$

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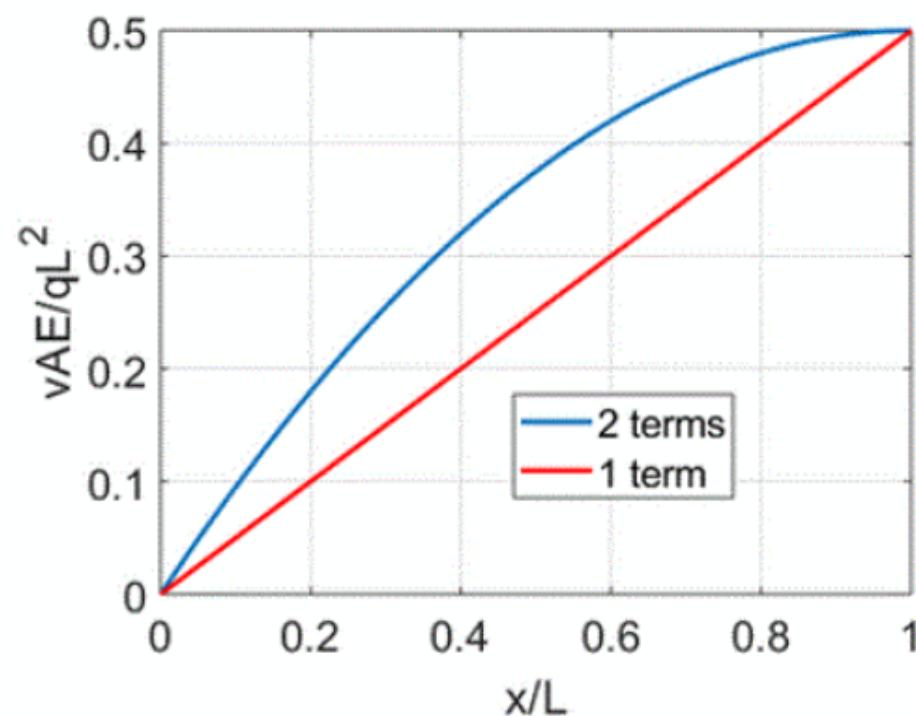
Try better guess  $a_1 + a_2 x + a_3 x^2$

- have MATLAB do algebra

```
syms v x a0 a1 a2 L E A q PI
v = a0 + a1*x + a2*x^2; % Approximation for displacement
eq1 = subs(v,x,0)==0; % Boundary condition v=0 at x=0
a0sol = solve(eq1,a0); % Solve for a0 (in terms of the other as)
v = subs(v,a0,a0sol); % Eliminate a0
PI = int(E*A/2*diff(v,x)^2 - q*v ,x,[0,L]); % Potential energy
eq2 = diff(PI,a1)==0; % d PE/da1 = 0
eq3 = diff(PI,a2)==0; % d PE/da2 = 0
[as1,as2] = solve([eq2,eq3],[a1,a2]); % Solve the equations
v = subs(v,[a1,a2],[as1,as2]) % Substitute solution into v
```

v =

$$\frac{Lqx}{AE} - \frac{qx^2}{2AE}$$



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What is the correct solution

Solve governing eq's exactly

(1) Equilibrium (method of sections)

$$\sigma A = q(L-x)$$

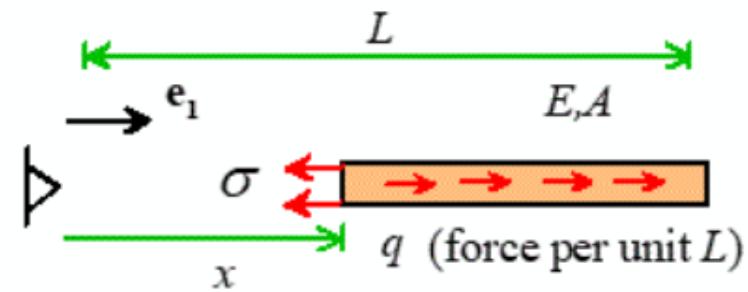
$$\Rightarrow \sigma = q(L-x)/A$$

$$(2) \text{ Stress-strain law} \quad \varepsilon = \sigma/E = q(L-x)/AE$$

$$(3) \text{ Strain-displacement} \quad \varepsilon = \frac{dy}{dx} \Rightarrow u = \int_0^x \varepsilon dx$$

$$\Rightarrow u = \frac{q_L x}{AE} - \frac{qx^2}{2AE}$$

2 terms gives exact sol



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In general if approximation can describe  
exact solution with finite # terms  
it will give the exact sol

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